Modelling multi-phase flows in Nuclear Decommissioning using SPH

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Abstract

Modelling multi-phase flows in Nuclear Decommissioning using SPH
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Doctor of Philosophy
University of Manchester
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This thesis presents a two-phase liquid-solid numerical model using Smoothed Particle Hydrodynamics (SPH). The scheme is developed for multi-phase flows in industrial tanks containing sediment used in the nuclear industry for decommissioning. These two-phase liquid-sediments flows feature a changing interfacial profile, large deformations and fragmentation of the interface with internal jets generating resuspension of the solid phase. SPH is a meshless Lagrangian discretization scheme whose major advantage is the absence of a mesh making the method ideal for interfacial and highly non-linear flows with fragmentation and resuspension. Emphasis has been given to the yield profile and rheological characteristics of the sediment solid phase using a yielding, shear and suspension layer which is needed to predict accurately the erosion phenomena.

The numerical SPH scheme is based on the explicit treatment of both phases using Newtonian and non-Newtonian Bingham-type constitutive models. This is supplemented by a yield criterion to predict the onset of yielding of the sediment surface and a suspension model at low volumetric concentrations of sediment solid. The multi-phase model has been compared with experimental and 2-D reference numerical models for scour following a dry-bed dam break yielding satisfactory results and improvements over well-known SPH multi-phase models. A 3-D case using more than 4 million particles, that is to the author’s best knowledge one of the largest liquid-sediment SPH simulations, is presented for the first time.

The numerical model is accelerated with the use of Graphic Processing Units (GPUs), with massively parallel capabilities. With the adoption of a multi-phase model the computational requirements increase due to extra arithmetic operations required to resolve both phases and the additional memory requirements for storing a second phase in the device memory. The open source weakly compressible SPH solver DualSPHysics was chosen as the platform for both CPU and GPU implementations. The implementation and optimisation of the multi-phase GPU code achieved a speed up of over 50 compared to a single thread serial code. Prior to this thesis, large resolution liquid-solid simulations were prohibitive and 3-D simulations with millions of particles were unfeasible unless variable particle resolution was employed.

Finally, the thesis addresses the challenging problem of enforcing wall boundary conditions in SPH with a novel extension of an existing Modified Virtual Boundary Particle (MVBP) technique. In contrast to the MVBP method, the extended MVBP (eMVBP) boundary condition guarantees that arbitrarily complex domains can be readily discretized ensuring approximate zeroth and first order consistency for all particles whose smoothing kernel support overlaps the boundary. The 2-D eMVBP method has also been extended to 3-D using boundary surfaces discretized into sets of triangular planes to represent the solid wall. Boundary particles are then obtained by translating a full uniform stencil according to the fluid particle position and applying an efficient ray casting algorithm to select particles inside the fluid domain. No special treatment for corners and low computational cost make the method ideal for GPU parallelization. The models are validated for a number of 2-D and 3-D cases, where significantly improved behaviour is obtained in comparison with the conventional boundary techniques. Finally the capability of the numerical scheme to simulate a dam break simulation is also shown in 2-D and 3-D.
Declaration

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Nomenclature and Glossary

The following list of symbols and abbreviations are the ones used throughout this thesis. Any other notation will be introduced in the document when used locally.

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<th>Definition</th>
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</tr>
<tr>
<td>$A$</td>
<td>Shifting free parameter</td>
</tr>
<tr>
<td>$a_d$</td>
<td>Kernel normalisation constant</td>
</tr>
<tr>
<td>$a_v$</td>
<td>Artificial viscosity free parameter bulk</td>
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<td>EOS reference pressure</td>
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<tr>
<td>$c_v$</td>
<td>Volumetric concentration</td>
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<tr>
<td>$D$</td>
<td>Deformation Tensor</td>
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<tr>
<td>$D$</td>
<td>Diffusion coefficient</td>
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<tr>
<td>$Dx$</td>
<td>Particle spacing</td>
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<tr>
<td>$D_{\delta-SPH}$</td>
<td>$\delta$-SPH diffusion parameter</td>
</tr>
<tr>
<td>$f$</td>
<td>Field function</td>
</tr>
<tr>
<td>$f_{\text{serial}}$</td>
<td>Serial fraction of an algorithm</td>
</tr>
<tr>
<td>$g$</td>
<td>Gravity</td>
</tr>
<tr>
<td>$H$</td>
<td>Height</td>
</tr>
<tr>
<td>$h$</td>
<td>Smoothing length</td>
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<tr>
<td>$i$</td>
<td>Interpolation particle</td>
</tr>
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<td>$I$</td>
<td>Unit matrix</td>
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<td>$I_D$</td>
<td>First invariant of the deformation</td>
</tr>
<tr>
<td>$I_{II_D}$</td>
<td>Second invariant of the deformation</td>
</tr>
<tr>
<td>$I_{III_D}$</td>
<td>Third invariant of the deformation</td>
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<tr>
<td>$j$</td>
<td>Neighbouring particles</td>
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<tr>
<td>$J_2$</td>
<td>Second invariant of the stress tensor</td>
</tr>
<tr>
<td>$m$</td>
<td>Mass</td>
</tr>
<tr>
<td>$m$</td>
<td>Herschel-Bulkley-Papanastasiou stress growth</td>
</tr>
<tr>
<td>$Ma$</td>
<td>Mach number</td>
</tr>
<tr>
<td>$N$</td>
<td>Number of particles</td>
</tr>
<tr>
<td>$n$</td>
<td>Herschel-Bulkley-Papanastasiou log law</td>
</tr>
<tr>
<td>$P$</td>
<td>Pressure</td>
</tr>
<tr>
<td>$Pe$</td>
<td>Peclet number</td>
</tr>
<tr>
<td>$r$</td>
<td>Distance magnitude</td>
</tr>
<tr>
<td>$R$</td>
<td>Shifting vector</td>
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</tbody>
</table>
\( R_c \) Gas constant
\( Re \) Reynolds number
\( S \) External forces
\( t \) Time
\( T \) Temperature
\( T_d \) Number of threads
\( T^d \) The number of parallel threads
\( u \) Velocity
\( u_{max} \) Maximum velocity
\( V \) Volume
\( \nu \) Kinematic viscosity
\( W \) Smoothing kernel function
\( x \) Cartesian position
\( \alpha \) Pressure related Coulomb Parameter
\( \beta_v \) Artificial viscosity free parameter shock wave
\( \Gamma \) polytropic index
\( \Delta \) Dirac delta function
\( \delta_d \) \( \delta \)-SPH free parameter
\( \varepsilon \) Strain rate tensor
\( \kappa \) Cohesion related Coulomb Parameter
\( \mu \) Viscosity (dynamic)
\( \mu_p \) Physical dynamic viscosity
\( \mu_t \) Eddy turbulent viscosity
\( \Pi_{ij} \) Artificial viscosity
\( \rho \) Density
\( \sigma \) Total stress tensor
\( \tau \) Viscous stress tensor
\( \phi \) Repose angle of sediment

<table>
<thead>
<tr>
<th>Abbreviations</th>
<th>Description</th>
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<tbody>
<tr>
<td>ALE</td>
<td>Arbitrary Lagrangian Eulerian</td>
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<tr>
<td>ALU</td>
<td>Arithmetic logic unit</td>
</tr>
<tr>
<td>API</td>
<td>Application Programming Interface</td>
</tr>
<tr>
<td>API</td>
<td>Application Program Interface</td>
</tr>
<tr>
<td>CAD</td>
<td>Computer aided design</td>
</tr>
<tr>
<td>CFD</td>
<td>Computational fluid dynamics</td>
</tr>
<tr>
<td>CFL</td>
<td>Courant-Friedrich-Levy</td>
</tr>
<tr>
<td>CPU</td>
<td>Central Processing Unit</td>
</tr>
<tr>
<td>CUDA</td>
<td>Compute Unified Device Architecture</td>
</tr>
<tr>
<td>DEM</td>
<td>Diffuse Element Method</td>
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<tr>
<td>DP</td>
<td>Drucker Prager</td>
</tr>
<tr>
<td>EFG</td>
<td>Element Free Galerkin method</td>
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<td>EOS</td>
<td>Equating of State</td>
</tr>
<tr>
<td>Abbreviation</td>
<td>Description</td>
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<tr>
<td>FDM</td>
<td>Finite Differences Method</td>
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<tr>
<td>FEM</td>
<td>Finite Element Method</td>
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<tr>
<td>FPGA</td>
<td>Field-programmable gate array</td>
</tr>
<tr>
<td>FVM</td>
<td>Finite Volumes Method</td>
</tr>
<tr>
<td>GPGPU</td>
<td>General-purpose computing on graphics processing units</td>
</tr>
<tr>
<td>GPU</td>
<td>Graphic Processing Unit</td>
</tr>
<tr>
<td>GVF</td>
<td>Generalised Viscoplastic Fluid</td>
</tr>
<tr>
<td>HAL</td>
<td>Highly Active Liquor</td>
</tr>
<tr>
<td>HAST</td>
<td>Highly Active Storage Tanks</td>
</tr>
<tr>
<td>HDL</td>
<td>Hardware Description Language</td>
</tr>
<tr>
<td>HLW</td>
<td>High Level Waste</td>
</tr>
<tr>
<td>HPC</td>
<td>High Performance Computing</td>
</tr>
<tr>
<td>I/O</td>
<td>Input/output</td>
</tr>
<tr>
<td>Intel MIC</td>
<td>Intel Many Integrated Core Architecture</td>
</tr>
<tr>
<td>ISPH</td>
<td>Incompressible SPH</td>
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<tr>
<td>LUST</td>
<td>Local Uniform Stencil</td>
</tr>
<tr>
<td>MC</td>
<td>Mohr-Coulomb</td>
</tr>
<tr>
<td>MD</td>
<td>Molecular dynamics</td>
</tr>
<tr>
<td>MLPG</td>
<td>Meshless Local Petrov-Galerkin method</td>
</tr>
<tr>
<td>MPI</td>
<td>Message Passing Interface</td>
</tr>
<tr>
<td>MWS</td>
<td>Mesh-free Weak-Strong form method</td>
</tr>
<tr>
<td>NNL</td>
<td>National Nuclear Laboratory</td>
</tr>
<tr>
<td>OpenMP</td>
<td>Open Multi-Processing</td>
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<tr>
<td>PCI</td>
<td>Peripheral Component Interconnect</td>
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<tr>
<td>PIM</td>
<td>Point Interpolation Method</td>
</tr>
<tr>
<td>RAM</td>
<td>Random access memory</td>
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<tr>
<td>RKPM</td>
<td>Reproduced Kernel Particle Method</td>
</tr>
<tr>
<td>SIMD</td>
<td>Single instruction, multiple data</td>
</tr>
<tr>
<td>SIMT</td>
<td>Single instruction, multiple-thread</td>
</tr>
<tr>
<td>VPS</td>
<td>Vortex Particle Simulations</td>
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<tr>
<td>WCSPH</td>
<td>Weakly Compressible SPH</td>
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</table>
Chapter 1

1. Introduction

1.1. Background

Problems that involve two or more phases, highly non-linear deformations and free-surface flows are a common occurrence in applied hydrodynamic problems in mechanical, civil and nuclear engineering. The two-phase liquid-solid interaction is a typical problem in hydraulics and more specifically flow-induced erosion. In nuclear engineering, sediment resuspension and scouring at the bottom of industrial tanks is used widely for mixing, filtration, heat-generating sediment flows and reservoir scouring. However, liquid-sediment problems are not restricted to nuclear engineering, other examples include port hydrodynamics and ship induced scour, wave breaking in coastal applications and scour around structures in civil and environmental engineering flows.

Figure 1.1. The storage of liquid high level waste tank internal configuration at Sellafield, UK [87].
A real life engineering application is being developed for the U.K. nuclear industry by the National Nuclear Laboratory Legacy Waste, Decommissioning & Disposal R&D program (NNL), U.K. where the resuspended sediment is agitated in industrial tanks by rapidly-varying flows with internal jets. A typical sediment resuspension tank is shown in Figure 1.1. These subaqueous sediment scouring flows are induced by rapid inflow creating shear forces at the surface of the sediment which cause the surface to yield and produce a shear layer of suspended particles at the interface and finally sediment suspension in the fluid. The current application is very difficult to treat with traditional Computational Fluid Dynamics (CFD) approaches such as Finite Volumes Methods (FVM) and Finite Element Methods (FEM) due to the fluid-sediment interface, the highly non-linear deformation of the sediment and entrainment of the sediment particles by the fluid phase with additional heat effects. These difficulties require alternative simulation techniques. In the past two decades the novel Lagrangian approach Smoothed Particle Hydrodynamics (SPH) has emerged as a meshless method ideal for this application.

Resolving small-scale effects at the interface is essential to capturing complex industrial flows accurately with variable physical properties for each phase. The massively parallel architecture of Graphic Processing Units (GPUs) computing can significantly accelerate simulations to simulate fine particle resolutions required for such industrial applications in realistic time. The Lagrangian nature of SPH deems the method ideal for large deformation flows with non-linear and fragmented interfacial multiple continua and is the method of choice in this thesis.

1.2. Flows in Nuclear Decommissioning

When the nuclear fuel is spent in a nuclear reactor, the fuel becomes inefficient and no longer viable for cost effective operation of the reactor. Reprocessing separates potentially reusable parts of the fuel such as uranium and plutonium. Unusable fuels such as fission products emerge as a waste stream produced by the reprocessing are known as High Level Waste (HLW). The HLW in the form of Highly Active Liquor (HAL) generate sufficient amounts of heat, requiring cooling. A reliable cooling system is needed in the storage facility. HAL which is a concentrated solution of fission products in nitric acid comprised of a liquid and dense sludge component stored in Highly Active Storage Tanks (HASTs) after the reprocessing process. The waste of the reprocessing process produces the so-called raffinate which is impractical to store without treatment due to the large amount of radioactive
material it contains. After an evaporation process the raffinate is stored in HAST tanks. The HASTs tanks were commissioned in the 1970 in the U.K. with a capacity of 150 m$^3$ each. The diameter of a tank is 6 m with a height of 6 m each using seven internal cooling coils and sediment agitation systems. The target is to maintain a temperature in the nominal range of 50-60 C$^\circ$ and above 45 C$^\circ$ to avoid crystallisation. Cooling is applied by the cooling coils and agitation of the sludge by a 7 internal jet ballast rig. Under air pressure in a closed circuit the jet scours the base of the HAST and suspends solid products of the fission contained in the HAL. A schematic of the HAST is shown in Figure 1.2.

Figure 1.2. Schematic of the internal arrangement of a HAST [87].
A typical chart displaying the decay time of the spent fuel is shown in Figure 1.3. Throughout the active heat generating life of the HAL the HAST cooling coil system and jet ballast rig remains operational maintaining expected temperature levels in the tank. The jet ballast operation and performance is of significant importance since accumulation of sediments piles at the bottom of the tank could lead to possible failure at the bottom of the HAST due to localised hot-spots that increase the temperatures and therefore the corrosion rate of the HAST.

The aim is to empty the HASTs as part of the Post Operation Clean Out (POCO) phase which may cause a decrease to the effectiveness of the jet ballast as the liquor level reduces and therefore, reducing the scour ability of the jets. Hence, there is a need to investigate the resuspension of solids in different scenarios with a general two-phase liquid-sediment model in order to optimise the POCO process.

In this thesis, the scouring and suspension of sediment induced by rapidly varying flows that resemble a jet are being investigated by modelling the two-phase flows in a monolithic SPH scheme. This thesis is part of the National Nuclear Laboratory Legacy Waste, Decommissioning & Disposal R&D program and aims to enhance NNL’s existing multi-phase modelling capabilities.
1.3. Smoothed Particle Hydrodynamics

Numerical modelling has become an essential tool in many branches of science, engineering and applied science. The physical phenomena are described by a mathematical formulation of governing equations belonging to a realm of continua. Secondly, a technique must be devised to solve the governing equations that usually, due to the complexity of the system require some sort of numerical approximation. The numerical approximation or discretization techniques in combination with the advances of computing power have been dominated by mesh-based methods in CFD and other applied science field with FVM and FEM. These methods are Eulerian and have been used traditionally across the entire field of CFD modelling. Moreover, Eulerian mesh-based methods are mature, robust and well accepted by the scientific and industrial communities [9].

However, some limitations are inherent to the Eulerian description and mesh itself for problems that involve interior domain boundaries such as free-surfaces, interfacial flows and fragmentation [1]. These difficulties are intrinsic to the interconnected mesh which is fixed in space in an Eulerian continuum discretization sense. Contrary to mesh-based methods, Lagrangian mesh-free methods use a nodal description of the continuum without fixed interconnected mesh by arbitrarily distributed computational points.

Smoothed Particle Hydrodynamics is a Lagrangian mesh-free method originally developed for non-axisymmetric astrophysical problems in 3-D space that resemble classical Newtonian hydrodynamics by Lucy [133] and Gingold and Monaghan [70]. The last two decades have seen SPH applied in a variety of scientific fields ranging from astrophysics [111] to coastal engineering problems [47, 49, 181], fracture mechanics [14] and electro-magnetic field simulations [2, 66, 171]. The meshless particle (computational nodes), Lagrangian nature of SPH and its ability to approximate the continuous governing equations in problems involving large non-linear deformations with little effort makes SPH ideal candidate for a variety of flows. These include multi-phase continua with surface flows, highly non-linear deformations and fragmentation of the free surfaces and interfacial flows. Hence, as discussed in subsequent Chapters SPH is an ideal discretization scheme for multi-phase flows such as liquid-sediment interaction.
1.4. Objectives of the Thesis

The objective of this thesis is the development of a two-phase liquid-sediment numerical model based on the SPH formalism for the simulation of local scouring induced by rapid liquid flows. The numerical model is developed for use in CPU and GPU implementations in the Weakly Compressible SPH code (WCSPH) DualSPHysics [44]. DualSPHysics is a C++/CUDA based solver with pre- and post-processing capabilities that uses Graphic Processing Units (GPUs) to accelerate the numerical computations. The GPU hardware allows for large industrial problems to be modelled in realistic time and cost. However, the thesis is not only restricted to multi-phase problems. Historically, one of the most challenging elements of SPH has been the development of boundary conditions which are not intrinsic to the SPH formulation. A novel boundary condition is presented in this thesis aiming towards improving the SPH scheme and reducing the numerical errors that are associated with the wall boundary conditions.

The multi-phase model developed herein is not restricted to nuclear decommissioning flows. The goal for the numerical model is to be applicable to other scientific problems such as subaqueous debris flow and scour around structures under rapid flows. To achieve the aforementioned goals a summary of the key issues have been addressed:

- The use of an accurate and robust liquid phase model with the addition of density diffusion to avoid spurious pressures and shifting algorithm to avoid unphysical voids in the liquid phase
- The improvement of the current formulation of multi-phase liquid-soil models by investigating the yield characteristics of the sediment
- Use of constitutive models to represent accurately the rheological characteristics of the yielded surface and suspension of the sediment phase.
- Implementation and optimisation of a GPU code that accelerated the multi-phase simulation to realistic time limits and the reduction in computational cost
- Validation and verification of the GPU multi-phase implementation using a variety of well known 2-D and a 3-D case and comparison with other numerical multi-phase models
- Development of a new novel wall boundary condition
1.5. Outline of the Thesis

The remained of this thesis is organised as follows:

Chapter 2 reviews the available literature and recent advances of meshless methods specifically in the context of Smoothed Particle Hydrodynamics. In addition, a literature review regarding advances in multi-phase flows with emphasis to liquid-sediment flows is presented. Chapter 2 also includes a review of the recent advances in state-of-the-art hardware acceleration techniques.

Chapter 3 focuses on the theoretical and mathematical background of SPH with the mathematical description of the method that includes the integral representation, discrete approximation and kernel function. Chapter 4 then presents the discretization procedure followed in this work to solve the governing equations, the numerical implementation and other necessary sub-closure models.

Chapter 5 deals with the description and implementation of the multi-physics models for liquid-sediment flows with a detail description of the yield criteria constitutive equations and other sub-closure models. The liquid phase is modelled using state-of-the-art WCSPH approach using the Newtonian solver of DualSPHysics with the use of δ-SPH for smoothing the pressure field of both phases. More importantly, shifting algorithms have been applied to the liquid phase avoiding the use of pressure and velocity smoothing using the XSPH approach. In addition, a standard Smagorinsky algebraic eddy viscosity turbulent model has been used in the liquid and yielded sediment phase. The sediment phase has been investigated in depth in two main regions. The first accounts for the yield surface of the sediment induced by the liquid. This work looks at a variety of yield criteria and compares results between the Mohr-Coulomb and the Drucker-Prager yield criterion and their applicability to the scouring problem though rapid flows. In addition, the yield criteria are reformulated as a constitutive equation and a comparison of the results is performed. Secondly, a variety of non-Newtonian constitutive equations are investigated based on a standard Bingham constitutive model with shear thinning or shear thickening characteristics and stress growth control for low and high stress states.

This is followed by Chapter 6 that discusses the hardware acceleration aspect of this thesis, the different architectures available but most importantly the GPU implementation in DualSPHysics. The GPU development remains a key area to this research and SPH in
general. The model has been implemented in the GPU DualSPHysics solver yielding significant speed up by optimising the multi-phase model to suit the GPU architecture.

Chapter 7 presents the validation and verification of the liquid-sediment model using a variety of 2-D and one 3-D case. A number of 2-D test numerical experiments have been compared not only with reference data but in addition, other SPH numerical models with satisfactory results. Moreover, a 3-D case using more than 4 million particles that is to the authors best knowledge one of the largest liquid-sediment SPH simulation, for the first time is performed with SPH.

Chapter 8 presents the wall boundary condition formulation, validation and discussion in a separate content to the multi-phase model. A novel boundary condition extending the work of Vacondio et al. [197] is presented that attempts to reduce the error associated with the wall boundary conditions in SPH by reducing the zeroth and first order moment error of the kernel and its derivative and therefore, restoring approximate zeroth and first order consistency in the wall boundary.

Finally, Conclusions and Future work are presented in Chapter 9.
Chapter 2

2. Literature review

2.1. Introduction

This Chapter provides a concise description of meshless computational schemes, discusses recent advances of SPH, its application to multi-phase flows and hardware acceleration techniques. In particular, a short description of Lagrangian and meshless particle methods is provided followed by the background and main advances of SPH discretization scheme and its applicability to fluid dynamics and multi-phase flows. An in-depth investigation on the up-to-date advances of multi-phase flows and more specifically sediment scour and resuspension due to rapid and fast varying flows is reported. Moreover, recent developments on hardware acceleration using Central Processing Units (CPUs) and co-processors such as Graphic Processing Units (GPUs) architectures are presented.

2.2. Meshless methods

Conventional mesh-based numerical methods such as Finite Differences, Volumes and Elements Methods (FDM, FVM and FEM respectively) have dominated the discretization schemes of numerical simulations such as fluid dynamics, solid mechanics and geotechnics. However, some inherent difficulties in some aspects of mesh-based methods can limit their application to problems that involve highly non-linear deformation such as free-surfaces and fragmentation and interior domain boundaries similar to interfacial flows [1].

These difficulties are intrinsic to the interconnected mesh which is fixed in space in an Eulerian continuum discretization sense. Tracking inhomogeneities, free surfaces, deformable and interfacial boundaries with non-linear violent kinematics within a fixed nodal frame is a formidable task without re-meshing techniques [1]. Re-meshing techniques can be cumbersome and time consuming in Eulerian and Lagrangian mesh based schemes [11]. The aforementioned limitations can be observed with multi-phase free-surface flows where the
deformation of the interface is non-linear and usually fragmentation occurs in violent hydrodynamic flows [148].

On the other hand, meshless methods and more specifically mesh-free particle methods such as SPH use a Lagrangian nodal description of the continuum avoiding the need to know explicitly the connectivity of the arbitrarily distributed nodes (or particles), large deformations and non-linear phenomena [119]. A good comparison between mesh- and meshless based methods is given by Agertz et al. [1].

Meshless methods use the node position in a combination with a nodal shape function to approximate the system of governing equations. Some meshless methods along with their approximation method are given in Table 1.

<table>
<thead>
<tr>
<th>Method</th>
<th>Method of Approximation</th>
<th>References</th>
</tr>
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<tbody>
<tr>
<td>Smoothed Particle Hydrodynamics (SPH)</td>
<td>Integral representation</td>
<td>Lucy, [134]</td>
</tr>
<tr>
<td>Finite point method (FPM)</td>
<td>Finite difference representation</td>
<td>Liszka and Orkisz [118]</td>
</tr>
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<td>Diffuse Element Method (DEM)</td>
<td>Moving Least Square (MLS) – Galerkin method</td>
<td>Nayroles et al. [159]</td>
</tr>
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<td>Element Free Galerkin (EFG) method</td>
<td>MLS approximation – Galerkin method</td>
<td>Belytschko et al. [12]</td>
</tr>
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<td>Reproduced Kernel Particle Method (RKPM)</td>
<td>Integral representation – Galerkin method</td>
<td>Liu et al. [128]</td>
</tr>
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<td>Free mesh method</td>
<td>Galerkin method</td>
<td>Yagawa and Yamada [213]</td>
</tr>
<tr>
<td>Meshless Local Petrov-Galerkin (MLPG) method</td>
<td>MLS approximation – Petrov-Galerkin method</td>
<td>Atluri and Zhu [8]</td>
</tr>
<tr>
<td>Point Interpolation Method (PIM)</td>
<td>Point interpolation – Galerkin and Petrov-Galerkin method</td>
<td>Liu and Gu [121]</td>
</tr>
<tr>
<td>Meshfree Weak-Strong (MWS) form</td>
<td>MLS, PIM, Collocation and Petrov-Galerkin method</td>
<td>Liu and Gu [122]</td>
</tr>
</tbody>
</table>

Table 1. Typical meshless methods listed in chronological order as presented by Liu and Liu [123].

In general, meshless methods use either a strong, weak or particle form of the governing equations with the exemption of some schemes that use a combination such as the MWS
[122]. A strong form method such as the FPM has the advantage of simplicity since the discrete system does not require an integration to obtain the discretized equations but accuracy and instability is a major drawback specifically when satisfying the Neumann condition [119]. Weak form schemes such as the RKPM are better suited to partial differential equations. The weak formulation tend to be stable and accurate mostly satisfying the Neumann condition since they use an integral operation to establish a discrete system of ordinary differential equations (ODE). Generally, the weak form is obtained through a Galerkin method or otherwise [119]. However, weak-form schemes tend to use a background local mesh for the integration of these weak forms that can be cumbersome and computational expensive [209]. Finally, a particle-form scheme uses a combination of collocation techniques and weak form integral representation. A representative example of the particle form or meshless particle methods is SPH. A local mesh is not required in SPH since the weak form operation is performed in the function approximation rather than the discrete system definition usually performed with Galerkin methods. A comprehensive review of meshless methods can be found in the book of Liu [119], Liu and Liu [123] and relevant work by Belytschko et al. [11]. In addition, more information on comparisons of strong and weak forms of meshless schemes has been conducted by Trobec et al. [193].

In this thesis, SPH has been selected due to its Lagrangian local interpolation formulation that combines a truly meshless method with a weak form making it ideal for multi-phase interfacial flows where phase discontinuities, interfacial fragmentation and free surfaces exist. Moreover, the explicit temporal integration and the local integral representation technique applied to SPH deem the method robust and accurate. Next, the SPH background and different variants within a fluid dynamics approach is discussed.

2.3. Smoothed Particle Hydrodynamics overview

2.3.1. Background of SPH

Smoothed particle hydrodynamics is a meshless particle method originally developed for continuum scale applications and initially applied in non-axisymmetric astrophysical problems in 3-D space that resemble classical Newtonian hydrodynamics by Lucy [133] and Gingold and Monaghan [70] in the 1970’s. SPH is still widely used in astrophysics [111] in simulations of galaxy formations [15], binaries stars [13], coalescence of black holes [190], etc., with popular SPH astrophysical codes such as GADGET-2 [189].
2.3.2. Early development of SPH

Early SPH formulations, derived from probability theory and statistical mechanics, did not conserve linear and angular momentum which is a challenge in fluid dynamics with conservation of mass and momentum (see Section 4.2 and 4.3). Gingold and Monaghan realised conservation of momentum was important in other fields such as fluid and solid dynamics and proposed an SPH conservative algorithm [71] which was later developed and applied in shock dynamics using an artificial viscosity term similar to Von Neumann-Richtmyer [208] to introduce viscous dissipation [150]. Monaghan [149] further developed the scheme by proposing the use of symmetric formulations that conserve momentum and improve the accuracy and stability of the scheme.

2.4. Applicability of SPH

The original SPH method developed by Lucy [133] and Gingold and Monaghan [70] was intended for modelling astrophysical problems which involve large perturbations, enormous variations in length and time and coupling with other astrophysical particle based methods that favoured SPH in astrophysics. The meshless particle characteristics of SPH and its ability to approximate the continuous governing equations involving large non-linear deformations with little effort makes SPH an ideal candidate for other scientific fields outside of astrophysics [151].

SPH has been applied to a vast range of problems in electro-magneto dynamics, solid and fluid mechanics, geotechnics, etc. A representative but far from complete list includes examples from electro-magnetic field simulations [2, 66, 171], fracture mechanics [14], metal forming and die casting [20, 41, 78], heat conduction [37, 97], fluid-solid interaction [7, 39, 187], coastal hydrodynamics with water wave impact, sloshing and overtopping [47, 49, 181].

In this work, sediment scour and resuspension of the sediment by liquors in industrial tanks require a combination of disciplines such as plastic flow [115, 131], environmental and geophysical flows [25-28, 31, 85, 107, 136, 217] and a variety of multi-phase flows [43, 69, 88, 90, 138, 153, 180, 195]. A more detailed literature review is given in Section 2.6 that discusses the recent advances and the applicability of SPH to multi-phase fluid-soil interaction.
2.5. SPH formulations for Fluid Dynamics

2.5.1. SPH variants

There are several variants of SPH in the literature such as the classical SPH [149], Godunov-type SPH [167], Arbitrary Lagrange Euler (ALE) SPH [206], Incompressible SPH (ISPH) [212] and methods that are closely related to SPH such as the FPM [118], RKPM [128], etc. Herein, a short description of these methods is provided in addition to the classical weakly compressible SPH (WCSPH) that is the method of choice for this thesis for reasons that will be explained shortly.

Parshikov et al. [167] developed an Godunov-type SPH to be used with shock wave discontinuities such as shock tubes by using the contact interaction between the SPH particles using approximate Riemann solutions for the discontinuities instead of the traditional particle pair-wise interactions without the use of the artificial viscosity formation to stabilise the domain. Vila et al. [206] developed an ALE scheme in an SPH formalism with a combination of Godunov-type finite difference method with the use of Riemann solvers to reduce the numerical noise in the pressure field and increase stability of the method reporting significant improvements for the pressure field and particle disorder.

Another popular variant of SPH is the ISPH [212]. ISPH imposes strict incompressibility by applying the Poisson equation using a projection method of Chorin [40] by either keeping a divergence-free velocity field [48], by keeping a density invariance [182] or based on combining both a divergence-free velocity field and a density-invariant field [89]. Until recently, most ISPH approaches suffered from particle instability and clumping altering the pressure field considerably. Lately, Xu et al. [212] applied the divergence-free velocity field approach using a particle shifting algorithm to reduce the particle instability error with irregular particle distributions. As a result, the pressure field was significantly improved. Particle shifting was further improved by Lind et al. [117] using a Fickian approach based on the particle concentration in addition to improvements on the treatment of free surfaces within the ISPH formalism. However, the implicit nature of ISPH with the Poison solver tends to make the method computationally expensive but noise-free pressure fields permit time steps that are approximately 10 times larger than in WCSPH. Also, the explicit treatment of the pressure at the free-surface may be cumbersome for violent fragmented free surfaces with further developments addressed by Skillen et al. [185].
2.5.2. Weakly compressible SPH

The classical SPH formulation relies on the local interpolation to express a field function such as velocity, pressure, viscosity, etc., using local quantities in discrete Lagrangian locations defined by a set of arbitrary particles. The gradients are calculated using a differentiable smoothing function by applying pair-wise interactions of particles identified by the use of the smoothing function area of influence [149]. Using a WCSPH approach in fluid dynamics, the pressure is computed from an equation of state (EOS) derived from thermodynamics laws. A typical WCSPH equation of state is the so-called Tait’s equation of state [10] which relates the pressure to density using the numerical speed of sound, the reference density of the fluid and the polytropic exponent on the density ratio. The equation of state does not enforce strict incompressibility, instead, a relative incompressibility is attained such that a small change in density produces large pressure variations with the density variations smaller than 1% of the reference density. To achieve the relative incompressibility (or weakly compressible flow) the numerical speed of sound is chosen to be on the order of a magnitude larger than the maximum bulk fluid velocity [73]. Pressure fluctuations due to the stiff equation of state, with the polytropic index usually set to 7 for incompressible fluids such as water, is often treated using zeroth and first order renormalisation techniques such as Shepard filter and Moving Least Squares (MLS) respectively as described by [43]. Lately, novel density diffusion methodologies, such as \( \delta \)-SPH [145], are applied directly to the mass conservation equation.

The major advantages of WCSPH over other variants such as ISPH is the regular particle distribution at low Mach numbers, the implicit treatment of the free surface where the pressure reduces to zero through the use of the equation of state, its applicability to interfacial flows at the interface [64] and its explicit temporal integration scheme which allows symplectic integrators to be used [73]. Since, the numerical speed of sound is used for the signal propagation, the numerical speed of sound is directly linked to the permissible time step in the simulation and therefore affects the computational cost explicitly [149]. At high Reynolds numbers, WCSPH density variations increase with the creation of unphysical voids that require a higher numerical speed of sound [111]. The development of particle shifting algorithms [117, 212] reduce the void formation in WCSPH at high Reynolds numbers and improve the pressure field of the flow [144]. It should also be mentioned that, from a computational point of view, the programming effort of WCSPH is considerably lower that the aforementioned methods.
Choice of SPH formulation
The Lagrangian nature of WCSPH and its applicability to free surface and interfacial flows through the equation of state deem WCSPH a strong candidate for flows that involve two or more phases and non-linear and fragmented interfaces. For that reason, WCSPH has been applied extensively to multi-phase flows such as geophysical flows, soil mechanics and fluid-solid interactions as already mentioned. Furthermore, the explicit formulation and pair-wise interactions of WCSPH makes the method suitable for use in massively parallel architectures such as co-processors and more specifically Graphic Processing Units (GPUs) as presented in this thesis.

2.5.3. Viscosity formulations
As already introduced, the artificial viscosity formulation of [150] is widely used in SPH due to its simplicity and low computational cost but is empirical and only loosely related to the physics of the problem. Instead, the empirical parameters are tuned to each application by relating the Reynolds number to the empirical parameters of [152] in fluid dynamics problems. However, other more rigorous formulations have been developed.

The main issue with the viscous formulation in SPH is mainly related to the shape of the second derivative of the smoothing function where the positive slope of the second derivative of the kernel is unstable in tension and negative unstable in compression [191]. The instability is manifested as particles clumping together in an SPH simulation. Morris [157] used a mixture of SPH and a finite differences approach to avoid the use of the second derivative of the kernel. He used a finite difference approximation in the velocity gradient by using a Jacobian transform and an SPH approximation for the viscous force. The viscous formulation conserves linear but not angular momentum and is only applicable to low Reynolds numbers and laminar flows [157].

An alternative formulation was developed by Monaghan-Cleary-Gingold [148] by using a methodology used in the heat equation to approximate the Laplacian resulting in a radial viscous force with respect to the velocity difference of two particles with an antisymmetric formulation that conserves linear momentum.

Another interesting formulation was proposed by Chaniotis et al. [34] using a re-meshing technique. Chaniotis et al. [34] used the second derivative of the smoothing function with a periodic re-initialisation of the position of the particles (by re-meshing the particles to an
orthogonal grid) to avoid the sensitivity of the second kernel derivative in particle disorder that leads to clumping and tensile instability. The re-meshing SPH technique produced satisfactory results but deemed SPH mesh-dependant. In addition, the computational cost of re-meshing every few time steps is considerable large.

Finally, a different formulation used in this thesis is derived directly from the viscous forces of the momentum equation without using the smoothing function second derivative. This formulation was used by Speith et al. [188] in astrophysical simulations but is not very popular in SPH with traditional fluid dynamics due to computational cost considerations. Nevertheless, recent developments in hardware acceleration deem the method attractive specifically in applications where viscous forces are important or large viscosity ratios exist between multi-phase flows such as the current application of fluid-sediment interfacial flows.

The method uses a double summation which is split in two steps. Firstly the velocity gradients are calculated with an SPH symmetric gradient summation and the viscous stress tensor is calculated using a constitutive equation (Newtonian or otherwise). Secondly the viscous stress tensor is summed to produce the deviatoric stresses of the momentum equation. Note, that the traditional symmetric SPH approach for calculating the strain rates and viscous terms does not violate linear and angular momentum conservation. To avoid \( N^2 \) interactions, firstly the strain rates and viscous stresses are calculated and saved, followed by the evaluation of the viscous term. This algorithm results in a \( 2N\log(N) \) interactions for the viscous forces but strain and stress information is readily available. In addition, non-Newtonian constitutive equations can be easily implemented which is advantageous for this work. An extensive comparison of viscosity models in SPH can be found in [74].

2.5.4. Particle instability

The Lagrangian particle nature of SPH leads to problems such as particle inconsistency resulting to a reduction of accuracy that has been researched extensively over the past years. Randles and Libersky [174] derived a normalisation formulation in an attempt to correct the density approximation in the interior domain. Similarly Chen et al. [36] developed the Corrective SPH (CSPH) by applying the kernel estimate into the Taylor series expansion with improvements in the interior and boundary domain. A Discontinuous SPH (DSPH) formulation was further developed by Liu et al. [125] to resolve problems with discontinuities such as shock waves. Dyka and Ingel [56] introduced stress points other than the SPH particles to calculate the stresses and zero energy problems. Methods such as the
RKPM and FPM are directly linked to SPH and the reader is directed to a review by Liu and Liu [120] for a more comprehensive list of tensile instability corrections and to work by Belytschko et al. [11] for a meshless method comparison.

2.5.5. Wall boundary conditions

Despite the recent success of SPH, in order to develop reliable numerical schemes some aspects of SPH models still require further research. Due to the intrinsic nature of kernel-based interpolation and to the Lagrangian approach, imposing boundary conditions in SPH is more challenging than in Eulerian grid-based models. Many early approaches adopted the repulsive force method [152] where the wall is described by particles which exert a repulsive short-range force similar to a Lennard-Jones potential force on fluid particles. In this way complex moving geometries can be readily discretized, but it is based on an empirical formulation and the problem of kernel truncation near the wall is not addressed.

Mirror or ghost particles as introduced by Randles and Libersky [174] are another widely used way to describe boundaries in SPH. The same idea was further extended by other authors using fixed-ghost [45], or multiple tangent methods [215] which can be used to discretize complex geometries. These methods have the drawback that particles can penetrate the wall and thus the mass conservation cannot be guaranteed.

Another class of numerical methods which have been used in SPH to discretize walls are the semi-analytical boundary conditions [58, 106, 143]. Kulasegaram et al. [106] proposed a variant of this method which introduces an additional term in the momentum equation in order to mimic the effect of the wall. This technique eventually uses an empirical function to approximate the force originating from variational principles. The idea was further developed in [58, 143] both for free-surface and confined flows. These methods have the advantage of restoring zero-consistency in the SPH interpolation but extending the method to complex 3-D geometries remains difficult [142]. Importantly, Mayrhofer et al. [143] showed that semi-analytical boundary conditions only approximately satisfy the skew-adjoint property which is a necessary criterion for energy conservation.

In an alternative but readily accessible approach, Ferrari et al. [59] proposed a local point symmetry (as opposed to ghost particles) method which is able to discretize arbitrarily complex geometries without introducing empirical forces. This approach is inherently capable of discretizing 2-D and 3-D complex geometries and to avoid kernel truncation...
effects. Recently the method was further enhanced in order to be applied to shallow water equations (SWEs) [197]. However, as will be shown in this thesis (see Chapter 8) when applied to the Navier-Stokes equations the method either performs poorly or fails completely for simple yet demanding free-surface flow problems. In the present work the method is modified in order to ensure approximate first-order consistency in a weakly compressible SPH scheme, without special treatment for arbitrarily complex geometries. It thereby retains the attractive simplicity of the original approach but extending its range.

Nevertheless, SPH boundary conditions remain an active area of research and constitutes as one of the Grand Challenges of the SPH community.

2.6. Modelling multi-phase gas-liquid flows with SPH

The local interpolation method in combination with the Lagrangian formulation in the absence of a predefined mesh allows SPH to model multi-phase interfacial flows without the need of complicated interfacial treatments. Also, the interaction and force exchange between each phase is relatively undemanding for fairly similar phases within the local approximation technique that SPH exploits [153].

Nevertheless, for different phases i.e. gas-liquid, performing a local approximation close to the interface can prove cumbersome depending of the physical properties and flow characteristics of each phase such as large density ratios that lead to different time steps on the temporal integration of two orders of magnitude [144], unphysical repulsion at the interface due to the large density ratio between the two phases and the high density gradients at the interface [43]. Other physical properties that influence the interface includes the viscosity ratio of the two phases that lead to large repulsion forces, generating instabilities at the interface [90]. Also, the rheological characteristics of each phase should be considered, since a multi-phase gas-liquid-sediment continuum may include Newtonian and non-Newtonian rheological models in addition to the fluid solid interaction. These types of multi-phase flows depend greatly of the shear characteristics of the flow at the interface and boundary of each phase [60, 90].

In the past decade, the development of multi-phase flows has been at the forefront of WCSPH research not only for fluid-soil interaction but for larger density ratios such as gas-liquid flows. Monaghan [153] initially used SPH to model a two phase mixture flow of a dusty gas using a void fraction approach to express the mixture of gas and dust particles to the
continuity and momentum equations. Nevertheless, the mixture interface with the gas had a low density ratio thus, explicit treatment of the interface was not necessary.

Another early notable approach simulating large density ratios for Newtonian fluids is the work by Colagrossi and Landrini [43] by the use of similar equations of state that are slightly modified for the gas to prevent mixing at the interface by adding an artificial pressure. Recently, Mokos et al. [144] applied the particle shifting algorithms of Lind et al. [117] and Skillen et al. [185] to improve the interface, reduce the physical gaps appearing in the gas phase and improve the pressure field of both phases.

A general multi-phase model was developed by Hu and Adams [90] by using the discrete volume of the particle instead of the density in the governing equations using a normalised smoothing function. With their approach, the momentum and continuity equation uses only the mass of each particle instead of the mass to density ratio. In addition, an inter-particle shear stress formulation is developed for flows dominated by shear forces. Results reported in [90] are promising but the scheme may prove cumbersome since tracking of the interface is required for the viscous forces. Grenier et al. [76] further developed the model mostly for gas-liquid flows by combining the approach of Hu and Adams [90] and Colagrossi and Landrini [43] with significant improvements.

In fluid-sediment interfacial flows, the density ratio for fully saturated flows is much smaller than that of gas-liquid; usually around 1.5. Consequently, the interface does not suffer from large numerical instabilities due to density. Nevertheless, the rheological characteristics of the two phases vary greatly. In addition, the shear forces of the liquid and the interaction with the sediment phase are predominant to the scour and sediment entrainment features.

For that reason a detailed literature survey of the sediment flow characteristics within SPH is given next in Section 2.7 with emphasis given to the yielding of the sediment surface by rapid liquid flow, the sediment rheological behaviour with scour and resuspension characteristics.

2.7. Modelling multi-phase liquid-sediment scour and resuspension with SPH

There is a great deal of interest in multi-phase flows and more specifically fluid-solid interaction in sea beds [219], debris flow [176], ship induced scour in harbours [195], reservoir flushing [138] and sediment scour and resuspension in industrial tanks [61].
The present focus of this thesis lies with the scour and resuspension of granular material of non-cohesive nature in industrial tanks agitated by rapid flows. The nature of the granular material can vary greatly between applications depending on the properties of the granular phase such as mean diameter of the granule distribution, shape of granule, bulk density and properties of the solid phase mixture such as porosity, concentration and saturation level.

Mesoscopic models have been used extensively in soil mechanics to describe the collisions of sediment grains from a statistical point of view [30, 67]. However, such models tend to be rather complicated and often require explicit knowledge of the properties of the granular phase. Herein, the surface failure of the sediment, the rheological behaviour of the scour layer and resuspension of the sediment is examined from a continuous macroscopic approach that is well suited to rheological models within SPH formalism.

2.7.1. Non-Newtonian sediment mixture models

Rodriguez-Paz and Bonet [187] used the Generalised Viscoplastic Fluid (GVF) model of Chen [35] to model the shear and plug flow of debris and avalanche failures as a non-Newtonian Bingham flow. In addition, they used the formulation proposed by Bonet and Lok [21] in a WCSPH formulation to correct the kernel interpolation. Furthermore, the Herschel-Bulkley model that is used in this work and other viscoplastic models are documented. The authors used an approach by Kanatani [100] to obtain the yield strength of the debris by using a reduction of the Drucker-Prager yield criterion that resembles the Mohr-Coulomb criterion.

To the best of the author’s knowledge, this is the first time a yield criterion has been used in SPH to simulate yielding of granular materials. Rodriguez-Paz and Bonet [187] simulated mainly dry granular avalanches with reasonable agreement to the experimental data for the front position of the avalanche and velocity of the avalanche front. In addition, the profiles of experimental field measurements compared with the numerical simulations were satisfactory. As the authors state, more advanced models could be used to capture the debris flow characteristics accurately. However, the simulations did not include a second phase such as liquid.

Further, Shakibaeinia and Jin [180] used the GVF model in an ISPH formalism for rapid scour and resuspension flows, where they applied a viscosity approximation for the two phases derived from dissipative fluid dynamics method at the interface in a manner similar to the laminar viscosity SPH approximation. In addition, the suspended sediment viscosity used
the Owens equation based on the volumetric concentration with good results in simulations over erodible dam breaks.

Similar to Rodriguez-Paz and Bonet [187], Hosseini et al. [86] examined non-Newtonian flows using ISPH with an explicit three step algorithm developed to account for the Bingham nature of the models tested. Hosseini et al. [86] tested the Bingham, power law and Herschel-Bulkley non-Newtonian models. The numerical results were compared with experimental results for a dam break simulation and the analytical solution for an annular couette flow. In addition, the authors produced results for a gravity current mud flow with good accuracy. Unfortunately, the current model did not explicitly treat each phase of the mud differently or as a mixture but rather as a simple one-phase flow.

Bui et al. [28] simulated erosion by a water jet on dry and fully saturated soil by developing a multi-phase model for liquid-soil interaction. In [28], the liquid is modelled using a WCSPH approach whereas the soil using an elastic-perfectly plastic material. In accordance to Rodriguez-Paz and Bonet [187] the stress state of the yielded soil is modelled using the Mohr-Coulomb yield criterion. The soil phase is modelled using the rheological characteristics of the Navier Stokes equations but the stress formulation is different. In their work, the soil is assumed to be elastic before yielding so that the pressure in the total stress tensor is calculated using a modified equation of state. This is achieved by relating the pressure to the bulk modulus and the density ratio. The deviatoric shear stresses of the elastic total stress tensor are calculated in a Newtonian approach by using the shear modulus of the Jaumann deformation rate. When plastic deformation occurs the shear stresses are calculated using the Mohr-Coulomb equation.

To model the fluid-soil interaction Bui et al. [28] superimposed the two different phase particles i.e. using a background particles approach. The motion of each phase is solved separately using its own SPH equations for the liquid and soil. The interaction between the phases occurs by using the seepage force in the momentum equation for each phase. Limited results were presented in this paper but future developments of this model demonstrate its applicability to multi-phase liquid-soil flows in [27] and embankment failures [25]. The disadvantage of this method is superimposing the two phases that increases the number of particles on the saturated soil considerably and impacts the computational cost. Moreover, the equation of state in the elastic region may lead to unphysical pressure variations if the bulk modulus has very large values. Furthermore, as stated by Bui et al. [28] the plastic
deformation constitutive model is simplistic and more accurate constitutive models should be used.

A similar approach is also used by Sakai et al. [179] to simulate seepage of soil for erosion of soil structures near liquids and failure of dykes. Sakai et al. [179] used the same superposition procedure for the two phases and coupled the soil and liquid using the frictional forces between the two phases through the Darcy law using the seepage force. Shao [182] used an ISPH model to solve the Navier Stokes equations for wave interactions through a porous media although in this work the porous media is a boundary and not moving granular material. Similar terms on the momentum representing the resistance force acting on the liquid by the porous media are reported in this paper with good agreement with the experimental and analytical solutions.

In addition, Bui et al. [26] replaced the simplistic plastic behaviour of the Mohr-Coulomb material by using an associated and non-associated plastic flow rule based on the Drucker-Prager model. The advantage of this approach combined with an elastic model based on Hooke’s law is the absence of the equation of state used previously by early researchers [115]. Instead, the pressure is calculated directly from the soil constitutive equation. In the work of Bui et al. [26], the elastic Hooke’s law and plastic flow rule based on the Drucker-Prager model was combined in the total strain rate tensor yielding a constitutive model for the total stress tensor through a plastic multiplier that defines the state of the soil, i.e. elastic or plastic loading/unloading. This method removed some tensile instability that appeared in previous attempts when the soil phase was unloading. The model produced realistic results for soil collapse of dry soil.

A similar approach was used to simulate soil-structure interaction through large deformation of geo-material using the Von Mises yield criterion for the structure and the elastic-plastic model for the soil. The interaction between the soil and the structure is modelled using Lenard-Jones repulsive forces described by Bui et al. [29]. Yet, the associative and non-associative plastic flow rule model can be cumbersome for multi-phase saturated and partly saturated soil flows that include resuspension by a liquid with large computational penalties.

However, explicit liquid-soil scour and resuspension in SPH has not yet been addressed in this literature review with the exemption of the ISPH model of Shakibaeinia and Jin [180] and the seepage force model of Bui et al. [28]. The reason is the importance of the
constitutive equations in the shear layer characteristics of the saturated soil phase. Next the liquid-soil scour and resuspension in SPH is considered.

2.7.2. Multi-phase liquid-sediment scour modelling in SPH

Recently, Sibilla [184] used the Exner equation to simulate the local scour caused by a 2-D horizontal wall jet on a non-cohesive granular bed downstream of a solid protection apron. The reasoning for the bed evolution model choice was justified by the physical characteristics of the flow and the relatively steady flow at steady state in combination with the long timescales of the slow varying bed profile. The Exner equation in conjunction with the Meyer-Peter-Muller formula which was used to evaluate the flow rate of the granular material was used by applying a finite difference scheme with explicit time integration. Results reported in this work were sufficiently accurate considering the complexity of the flow.

Falappi et al. [57] used the Mohr-Coulomb criterion to model scour in reservoir flashing by using the Newtonian constitutive equation in a pseudo-Newtonian approach. Falappi et al. replaced the viscosity of the fluid by the effective viscosity derived from the Mohr-Coulomb criterion as the ratio of the yield strength over the deformation invariant. The model was applied to a reservoir with sediment deposited at the bottom of the tank during the opening of an outlet, to capture the scour profile by the liquid flow. Numerical results were compared with the experimental model with reasonable agreement for the sediment profiles. This formulation has been the starting point for many researchers in scour using SPH for fast, violent unsteady flows including this thesis.

Manenti et al. [138] compared the Mohr-Coulomb pseudo-Newtonian approach of Falappi et al. [57] with the Shield’s criterion for the same experimental application. In addition, Manenti et al. [138] was the first to use the sediment skeleton pressure in the Mohr-Coulomb yield criterion. However, the technique required tracking the interface which can be cumbersome. The Shield’s criterion in conjunction with the Van Rijn relationships [201] were solved using an iterative process to determine the friction velocity (related to the hydrodynamic bottom shear stress). A logarithmic mixing length velocity profile was also used in the turbulent layer at the interface since the spatial discretization was under-resolved. Manenti et al. [138] demonstrated that for this specific application the Shield’s criterion predicted accurately the scour profile of the sediment phase. The Shield’s criterion has been used extensively in mesh based methods with success for currents and sea beds and river beds.
whereas the Mohr-Coulomb approach is well suited to applications where yielding of the surface occurs due to rapid flows or impact of the liquid to the bed.

Recently, Leonardi and Rung [114] combined the Mohr-Coulomb pseudo-Newtonian approach with the Shield’s parameter in an attempt to predict the yielding of the surface due to fast varying flows and the sediment erosion at the surface by the turbulent shear layer. Their approach yielded good results but a logarithmic mixing length velocity profile was not applied and it is likely that the turbulent shear layer was under resolved. Nevertheless, the approach may be applicable to a variety of applications.

Ulrich et al. [195] developed a scour and resuspension multi-phase model for ship induced scouring near ports. The model makes use of the Mohr-Coulomb approach for predicting the yielding of the sediment bed with a water-soil suspension model based on the Chezy-relation using piecewise linear interpolations between the soil, liquid and critical maximum viscosity for the suspension viscosity of the sediment. The model [194, 196] was further enhanced with an elastic branch for the un-yielded region based on Hooke’s law and a novel partly saturated formulation based on Darcy law. The partly saturated model accounted for seepage and capillary forces similar to an approach by Lenaerts [112]. The advantage of the model stems from the novel formulation for the suspended sediment and the capability of dealing with partly and fully saturated soils without using the superposition method of Bui et al. [25, 28]. However, the elastic stress formulation and the second derivatives involved in the diffusion of the pore water pressure may be problematic in SPH. Nevertheless, it is an important development within the SPH scour and resuspension methodology.

Similar to Bui et al. [28], Zanganeh et al. [216] used the superposition method, but instead of solving for an elastic-plastic sediment region, a soft contact model was used based on a spring-dashpot system between the contacting particles for the soil phase to simulate scour around a pipe in the seabed based on a collision mesoscopic approach.

It is evident from this literature review that SPH is well suited to multi-phase liquid-soil flows. Most work in this field includes a yield function to predict the yielding of the surface and a constitutive model at high stress states. In addition, some researchers use an elastic region to resolve the low stress state of the soil. The suspension formulation varies depending on the sediment transport application. This thesis will present a new development with emphasis on the yielding characteristic, constitutive model and suspension of the sediment.
These multi-phase flows tend to be computationally expensive since the shear forces at the interface must be treated explicitly, often involving two or more rheological models with a large amount of data storage required. Consequently, hardware acceleration is necessary for real-life engineering applications that require fine discretization in order to capture non-linear high order flows. Next, a short literature review on hardware acceleration within SPH is presented.

2.8. Hardware acceleration in SPH

The local interpolation procedure of SPH using arbitrary Lagrangian discrete points within an area of influence defined by the smoothing function increases the computational cost of the scheme in comparison with traditional mesh-based methods. For instance, in a Cartesian 2-D finite volume scheme each cell has 4 neighbours whereas in SPH the number of nodes (or particles) depends on the size of influence of the smoothing function which usually includes 21 particles for a typical smoothing length with uniform particle distribution (based on a Wendland kernel with a smoothing length of $2h$ and smoothing coefficient of 1.3 as used in this thesis, see Section 3.5). In addition, the number of particles within the smoothing function continually changes, since particles are allowed to move in or out of the area of influence.

Since SPH tends to be computationally intensive [44], spatial convergence in SPH is hard to achieve for large industrial applications with complex geometries and physics. In addition, in 3-D the number of particles increases dramatically from $N^2$ to $N^3$. Therefore, SPH practitioners may not reach spatial convergence due to computational cost limitations [54].

The simulation of multi-phase flows with gas, liquid or solid phases tend to be more expensive than a single phase flow in SPH. Consider a multi-phase flow where the numerical speed of sound of the gas is significantly larger than the liquid; Mokos et al. [144] reported time steps for the gas phase two order of magnitude smaller than the liquid. In liquid-soil flows, the interface and the viscous formulation in combination with the extra Courant-Fredrich-Levy condition for viscous forces results in computational penalties. Moreover, due to the physical domain, multi-phase flows tend to occupy more space in the interior domain and thus more particles are needed. Lately, particle splitting and coalescing [198] in areas of interest may decrease the computational cost by an order of magnitude but nonetheless SPH is still computationally demanding.
Since central processing units (CPUs) clock speed has not increased recently due to high power consumption and other technical and manufacturing reasons, parallel computing has been gathering momentum the past two decades [68]. CPUs and hardware acceleration tend to evolve from a serial execution approach to a parallel within the CPU chip by increasing the number of cores [93] or by parallelising many cores to massively parallel architectures [161]. Currently, hardware acceleration is either achieved by the use of large clusters of CPUs or by the use of co-processors. Next, the CPU and co-processors based hardware acceleration within SPH is presented.

2.8.1. CPU-based acceleration in SPH

Today’s modern computers all tend to possess some degree of parallelism incorporated in the CPU chip with a small number of cores with each chip (up to eight cores) [93]. Traditionally single CPU parallelisation is achievable with the Open Multi-processing protocol (OpenMP) (see Section 6.2.2). Although OpenMP tends to be efficient for small computations within a single PC or workstation, it does not provide the means for simulation of large industrial applications requiring 10-100 million of SPH particles.

Massive parallelism can be achieved by connecting several CPUs through a network and by using high speed protocols such as infiniband for chip communication. The system communication can be achieved through the Message Passing Interface (MPI) libraries (see Section 6.2.2) creating a large cluster. Such systems have been used extensively in SPH initially in astrophysical simulations [19, 50] with recent codes simulating several million of particles [189, 202].

In addition, the use of MPI and large clusters has been used in solid mechanics [202], fluid dynamics and free surface flows [59] with parallel MPI codes (e.g. Parallel-SPHysics [72]). However, there are significant drawbacks since the use of large supercomputer clusters are expensive with high energy consumption.

In the last few years, different massively parallel architectures have become increasingly popular. These architectures are based on multiprocessors located physically off the CPU chip called co-processors. Next, the main co-processors used in SPH are briefly presented with emphasis on the GPUs architecture and applicability to SPH.
2.8.2. Co-processors based acceleration in SPH

In scientific computing, and more specifically CFD modelling, the cost of floating operation per second (flops) per watt of energy is becoming increasingly important. As an alternative to the high numerical cost of CPU based clusters, co-processors are now used widely in CFD. Co-processors are massively parallel architectures located beside a CPU chip to perform arithmetic, logical operations, etc., accelerating the algorithm performance. Co-processors manage most numerical operations reducing the load of CPUs using parallel instructions. In addition, the low cost and low power requirements make co-processors an ideal alternative. A detailed description of the hardware architecture is given in Section 6.2.2.2. The relative high computational cost of SPH and the pair-wise interpolation method with an explicit temporal integration scheme similar to $n$-body simulations make SPH an ideal candidate for such hardware platforms. Some major advances in co-processing hardware are discussed next.

2.8.2.1. FPGA co-processors

FPGAs (Field-programmable gate arrays) co-processors use large amounts of logic gates with interconnect that can be programmed by the user to execute specific algorithms. The reconfigurable nature of FPGAs means that the chip interconnect is configured to a specific application rather than an algorithm to a chip [140].

The architecture has been applied to $n$-body simulations in the past [79] with considerable success. Recently, the FPGA accelerator has also been applied to astrophysics problems by Nakasato et al. [158] using SPH with reported speed up of 11 over traditional CPUs. Typical hydrodynamic problems in SPH include the work of Lienhart et al. [116].

However, programming the configurable hardware co-processor can be cumbersome [168] and many SPH practitioners tend to use different co-processor architectures with traditional based chips.

2.8.2.2. Xeon Phi co-processors

A new co-processor architecture was recently released by Intel Corporation [92]. The Xeon Phi co-processor is based on the x86 architecture used in CPU chips. Similar to the Graphic Processing Units (GPUs) Xeon Phi co-processor architecture is based on a many core architecture. In line with the CPU-based clusters, the Xeon Phi makes use of a shared memory design, allowing the developer to use tools developed for CPU cluster code use such as OpenMP and MPI [96].
The Xeon Phi co-processor is relatively new but its application to SPH has already been attempted. Examples can be found in astrophysics with the popular GADGET astrophysical code [24] with sufficient speed ups and good scalability, in hydrodynamics with a hybrid CPU/Xeon Phi implementation [52] and computer graphics [160]. However, despite the rapid development, the Xeon Phi architecture is fairly new with SPH codes starting to emerge.

Alternatively, the GPU co-processor architecture has been used in SPH hardware acceleration for a decade now with good success. Since this thesis hardware implementation is based on GPUs a more detailed survey on the hardware implementation and performance characteristics is given next.

2.8.2.3. GPUs acceleration
Graphic Processing Units (GPUs) have originated from video output and graphics rendering. In such applications, fast arithmetic operations are essential due to the nature of their function, i.e. real time rendering for graphics and output to screen. To achieve fast arithmetic operation and texture rendering, GPUs used massively parallel core architecture. Soon, it was realised that GPUs can be applied to other fields as a co-processors. That led to the development of general computing platforms such as the compute unified device architecture (CUDA) and OpenCL to handle the context, memory and execution management. A detailed description of the GPU architecture and CUDA is given in Section 6.3. The easy parallelism of SPH as an n-body simulation in pair-wise (see Section 6.2.1) makes SPH particularly suitable for GPUs with intensive arithmetic operations manner.

Early particle methods on GPUs date back to 2004 with the work of Kipfer et al. [102] and a SPH implementation by Amada et al. [3]. The authors used a hybrid approach with the main part of the code running on the CPU and intensive arithmetic operations to the GPU. However since it predates the general programming platform release (CUDA or OpenCL), the authors used OpenGL to implement the code on the GPU architecture employing a texture map approach within texture rendering algorithms developed for video processing. A texture map approached for scientific computing can be cumbersome due to the Cartesian 3-D nature of the texture memory arranged in a 2-D structure. Nevertheless, this early approach halved the computational time compared with the CPU approach.

Similar work was performed by Kolb and Cuntz [104] who realised memory transfer times can be minimised by running the code entirely on the GPU co-processor. The CPU portion of the code was used only for initialisation I/O operations. Their approach produced better speed
up than Amada et al. [3] that lead them to extend the code to 3-D. However, the 2-D texture memory structure and the rendering of the GPUs, force them to reduce the 3-D domain into 2-D slices. These 2-D structures interacted with each other through an interpolation thus increasing the numerical diffusion in the third dimension. Nevertheless, their approach was successful despite the difficulty of the GPU texture architecture.

Harada et al. [81] realised the potential arithmetic capability of GPUs with the work of Kolb and Cuntz [104] developing an balancing algorithm where each multi-processor would share its work balance in an MPI sense for particle based methods. In addition, a more efficient memory management algorithm was applied with dynamic allocation. On a later approach, the algorithm was applied to SPH with a neighbouring searching algorithm running exclusively on the GPU [82]. The authors performed extensive tests and showed a good scalability of the GPU in comparison to the CPU for large numbers of particles, also speed ups reported by the authors against a CPU based algorithm was on the order of a magnitude larger. Similar work has been performed by [214] for hydrodynamic problems.

However, GPUs only gained popularity as a general purpose scientific tool with the appearance of the CUDA platform from NVIDIA with the implicit handle of context, memory transfers within the GPU chips and execution of arithmetic operations and logic control.

With a mature CUDA platform and the earlier developments on GPUs and OpenGL [82], Hérault et al. [83] developed a GPU code based on an earlier serial Fortran code SPHysics of Gomez-Gesteira et al. [72]. The GPU code made use of the early developments such as the efficiency of GPU code running on the GPU entirely. Also, the authors performed many tests to maximise the use of the memory spaces on the GPU. These include the use of the constant memory for storing numerical constants and access latency of global memory. The authors experimented with one thread per particle and many threads for one particle, to observe that the former approach was faster on CPUs. A more detailed explanation for this approach is given in Section 6.2.1. Since GPUs are massively parallel structures, memory access and writing was also investigated. The authors concluded that performance can be achieved by using sequential and in-line memory address access.

Oger et al. [162] developed a hybrid directives based CPU-GPU code based on a parallel CPU code SPH-Flow. In their work, the CPU parallel code dealt mostly with low intensive arithmetic operations and flow control whereas the GPU code was used for the pair-wise
particle interactions. The authors used a space-filling curve algorithm to limit the particle interactions traditionally used with MPI implementations [77]. This MPI-CUDA hybrid code could potentially be extended to a multi-GPU approach with MPI communication between the GPUs. The speeds up from a single GPU card reported by the authors were almost half the computational time compared with 32 CPUs, which is a significant speed up. A similar approach has been used by Cercos-Pita et al. [33] using the OpenCL library with the AQUAagpusph solver. The numerical speed up reported by the authors is similar to the CUDA approach of Oger et al. [162].

More recently, DualSPHysics [44] was developed as a dual CPU / GPU code based on SPHysics [72]. The code shares similarities with the GPUSPH code of Hérault et al. [83]. DualSPHysics [44] comprises of a set of codes, split in a C/C++ and a GPU part. The GPU part runs the entire SPH computation avoiding the hybrid approach. In the absence of a GPU card the C/C++ algorithm is executed on the CPU using OpenMP.

DualSPHysics [44] uses most of GPU memory spaces with numerical constants on the constant memory space, large particle arrays in the global memory in a sequential and inline arrangement depending on their spatial position using the available Trust libraries of CUDA and the utilisation of the cache memory for fast transfer from the global to the registers memory space for new GPU cards. In addition, shared memory is being used for reduction calculations.

In a related paper by Domínguez et al. [55] the authors demonstrated the importance of the neighbour search algorithm in a GPU implementation. The authors showed that an efficient neighbour search algorithm can yield extra performance for large number of particles in a massively parallel architecture. DualSPHysics [44] uses the linked-list neighbour search algorithm that traditionally uses $2h$, the radius of the kernel support as the characteristic length to create a list of rectangular boxes for mapping the domain. The authors found that for large simulations in 3-D (more than 1 million particles), $h$, half of the radius is more efficient.

Domínguez et al. [54] performed extensive tests on CPU and GPU algorithms with high level of optimisation for both architectures. Also, the approach used on the code has some parts of I/O and case initialisation shared between the architectures allowed the authors for a direct comparison of the speed up of the SPH solver using different optimisation techniques for the
CPU and GPU platforms. The authors concluded that a speed up of 12.5 can be achieved for the GPU approach in comparison to a typical multithreaded CPU.

Further developments by Valdez-Balderas et al. [199] includes a multi GPU-MPI scheme. In their work, volume domain decomposition is used with MPI spreading the load to multiple GPUs. Similar with traditional MPI approaches particle data are being exchanged between MPI based CPUs using a halo on the linked-list box limits. The information is communicated between the CPUs and uploaded to the GPUs solver. The authors reported latency in the MPI communication that was further improved by Domínguez et al. [53] using in addition to the MPI approach, dynamic load balancing between GPU cards to account for heterogeneous GPU co-processors leading to a simulation of more than 1 billion particles over 64 GPU cards for a free-surface flow case.

A detailed description of the CPU and GPU DualSPHysics code [44] is presented in Section 6.4.

2.9. Concluding Remarks

This chapter has presented the historical background of SPH and multi-phase flows using SPH. The recent developments of multi-phase flows and the physical processes have been addressed and more specifically, the density ratio of the two phases and the viscous constitutive formulations that have been used in SPH to model such flows.

Liquid - sediment scour is dominated by the yield characteristics of the sediment due to the rapid flow of the liquid, the constitutive models for the dynamics of the sediment phase and the resuspension of the sediment by the fluid. SPH practitioners have developed models based on the specific application depending on the length scales and time scales of the phenomena.

Since these flows require explicitly resolving the interface and usually require larger numbers of particles due to the multi-phase nature, hardware acceleration is essential for industrial applications in 3-D. GPUs are ideal accelerators for SPH due to their massively parallel architecture and high arithmetic capabilities. Nevertheless, code optimisation for GPUs can be cumbersome and has attracted plenty of interest.

DualSPHysics is a GPU solver for WCSPH with highly optimised code for the CPU and GPU computing platform. In this thesis DualSPHysics has been chosen as the solver for the
implementation of the multi-phase model in the CPU and GPU code. In this thesis a multi-phase liquid-solid formulation is developed for scour and resuspension of the solid sediment phase using SPH.

The next Chapter presents the fundamental theoretical and mathematical background of SPH.
Chapter 3

3. Theory of SPH

3.1. Introduction

This chapter presents a description of the theoretical background of the SPH. This includes the particle nature of SPH and its Lagrangian nature, the mathematical basis of the method in a continuum and the discrete form that SPH uses to obtain approximate numerical solution of the governing equations by replacing the continuum with a set of discrete points. In addition, the smoothing kernel function and its fundamental properties are presented with some smoothing function examples.

3.2. Description of SPH method

SPH is a mesh-free Lagrangian discretization scheme where the continuum is discretized by a finite number of computational points. In a mathematical formalism, these are points of the discrete domain where quantities of the physical domain are interpolated. In the SPH formalism these interpolation points are called “particles”. Therefore, the continuum domain is discretized by a finite number of macroscopic volumes (or particles) which are defined in a continuum mechanics formalism representing the physical domain. Particles move in space according to the governing equations and are characterised by physical properties such as mass \( m \) and density \( \rho \) defining the volume \( V \) of the particles, pressure \( P \) with velocity \( \mathbf{u} \) and acceleration \( \mathbf{a} \) at their current position \( \mathbf{x} \) as shown in Figure 3.1 (a).

SPH uses an integral representation method in a weak form using a smoothing function over an interpolation domain that is called the support domain. In a discrete form the discretized quantity of particle \( p \) is achieved using a summation of the corresponding values of neighbouring particles. Therefore, the particle approximation depends on the local distribution of the neighbouring particles in the current position of the support domain at each time step as shown in Figure 3.1 (b). The above characteristics of the SPH method and in particular the local dependence on the support domain updated at every time step with the
addition of the Lagrangian formulation allows SPH to model problems with high deformation such as free-surfaces, two-phase flows and violent flows.

A detailed analysis in a mathematical formalism of the SPH method with the integral representation and particle approximation (or discretization) is presented next in detail.

Figure 3.1. Moving particle along a trajectory (a) with a velocity \( \mathbf{u} \) at position \( \mathbf{x} \) with a volume \( V \), (b) local distribution of particles within the support domain.

### 3.3. Integral representation

#### 3.3.1. Integral representation of a function

The basic principle of the SPH formulation is the integral representation of a spatial function \( f \) which may represent a numerical or physical variable defined over a domain of interest \( \Omega \) defined at a point \( \mathbf{x} \) that can be expressed by the convolution product of the function with the Dirac delta function \( \delta \) using the following expression [123]

\[
f(\mathbf{x}) = \int_{\Omega} f(\mathbf{x}') \delta(\mathbf{x} - \mathbf{x}') d\mathbf{x}',
\]

integrated over the domain \( \Omega \) with the Dirac delta function defined as

\[
\delta(\mathbf{x} - \mathbf{x}') = \begin{cases} 
1 & \text{if } \mathbf{x} = \mathbf{x}' \\
0 & \text{if } \mathbf{x} \neq \mathbf{x}'
\end{cases}
\]

The integral representation of the function \( f \) in Equation (3.1) is exact but uses an infinitesimally small domain (minimal support) and therefore for numerical reasons discussed
in Section 3.5 the Dirac delta function is not used even if it is an exact representation of the function \( f \). Consequently other smoothing kernel functions may be used of finite support with the integral approximation or kernel approximation according to

\[
    f(x) \approx \left\langle f(x) \right\rangle \equiv \int_{\Omega} f(x') W(x - x', h) dx',
\]

(3.3)

with \( h \) defined as the smoothing length that characterizes the size of the support domain of the kernel based on the smoothing length coefficient \( a \) and the initial particle spacing \( \Delta x \). Note that the parenthesis \( \langle \ldots \rangle \) represents an approximation as the Dirac delta function is not used. The kernel function is often chosen to be a smooth, isotropic and an even function with compact support such that

\[
    W(x - x', h) = 0 \quad \text{outside } \Omega.
\]

(3.4)

**A note on SPH accuracy**

Before continuing to the representation of the derivative in SPH formalism, the accuracy of Equation (3.3) can be determined by using a Taylor series expansion of \( f(x') \) around \( x \), where \( f(x) \) is differentiable. By substituting \( f(x') \) to Equation (3.3) it yields

\[
    \left\langle f(x) \right\rangle = \int_{\Omega} \left[ f(x) + (x'-x)f'(x) + O((x'-x)^2) \right] W(x - x', h) dx',
\]

(3.5)

or

\[
    \left\langle f(x) \right\rangle = f(x) \int_{\Omega} W(x - x', h) dx' + f'(x) \int_{\Omega} (x'-x)W(x - x', h) dx' + O((x'-x)^2).
\]

(3.6)

For a compact kernel support the following identity can be used

\[
    \int_{\Omega} W(x - x', h) dx' = 1,
\]

(3.7)

simplifying the first term on the right-hand side equation. Moreover, since the kernel is an even function with respect to \( x \) the second integral must be an odd function thus

\[
    \int_{\Omega} (x'-x)W(x - x', h) dx' = 0.
\]

(3.8)

Also, one can assume that the order of distance within the kernel is of \( h \) order and thus Equation (3.5) can be rewritten as

\[
    \left\langle f(x) \right\rangle = f(x) + O(h^2).
\]

(3.9)
This shows that the integral representation of a function $f$ is of second-order accuracy in space [149]. Consequently,

$$
\langle f(x) \rangle = \int_{\Omega} f(x') W(x - x', h) dx' + O(h^2). \tag{3.10}
$$

However, some conditions must be satisfied as will be discussed in Section 3.5 such as the aforementioned properties of compact support and the kernel being an even function. Next, the integral representation of the derivative of a function is demonstrated.

### 3.3.2. Integral representation of the derivative of a function

For a spatial derivative of the function $\nabla \cdot f(x)$ the approximation of Equation (3.10) reads [123]

$$
\langle \nabla \cdot f(x) \rangle = \int_{\Omega} \nabla \cdot f(x') W(x - x', h) dx' + O(h^2), \tag{3.11}
$$

where the divergence in the integral is operated with respect to $x'$. Equation (3.11) implies that the divergence of a function $f(x)$ can be approximated by the convolution product of the divergence of the function with the kernel. Since

$$
\nabla \cdot f(x) W(x - x', h) = \nabla \cdot [f(x') W(x - x', h)] - f(x') \cdot \nabla W(x - x', h), \tag{3.12}
$$

substituting in the integral of Equation (3.11) yields

$$
\langle \nabla \cdot f(x) \rangle = \int_{\Omega} \nabla \cdot [f(x') W(x - x', h)] dx' - \int_{\Omega} f(x') \cdot \nabla W(x - x', h) dx' + O(h^2). \tag{3.13}
$$

The first term on the right hand side of Equation (3.13) uses the spatial derivative applied directly to the function $f$. By using the divergence theorem this term can be rewritten as a surface integral of the domain $\Omega$, thus

$$
\langle \nabla \cdot f(x) \rangle = \int_{S} f(x') W(x - x', h) \cdot n dS - \int_{\Omega} f(x') \cdot \nabla W(x - x', h) dx' + O(h^2), \tag{3.14}
$$

where $n$ is the normal unit vector on the surface $S$. Due to the compact support of Equation (3.4) the surface integral vanishes such that

$$
\langle \nabla \cdot f(x) \rangle = -\int_{\Omega} f(x') \cdot \nabla W(x - x', h) dx' + O(h^2). \tag{3.15}
$$
Comparing Equation (3.11) with (3.15), the spatial derivative has shifted from the function $f$ to the kernel function, which it is known a priori, resulting in a weak formulation of the function $f$ reducing the consistency requirement of $f$.

The integral approximation must be related to the discretized domain. The discretized domain of particles can be related to the integral representation by using the particle approximation. Next, the particle approximation in SPH methodology is used to achieve the latter.

### 3.4. Discrete approximation

#### 3.4.1. Discrete approximation of a function

The continuous integral approximation of Section 3.3 is converted to a discretized form using a summation over the support domain relating the continuous form to the discrete domain. Equation (3.3) is approximated by

$$
\langle f(x) \rangle = \sum_{j}^{N} f(x_j) W(x - x_j, h) \Delta V_j + O(h^2),
$$

where the subscript $j$ denotes the $j^{th}$ particle such that $x_j$ denotes the location of particle $j$ summing for all particles $N$ within a kernel support of $2h$. The discrete volume $\Delta V$ represents to volumetric fraction of the interpolation point and can be written as

$$
\Delta V_j = \frac{m_j}{\rho_j}.
$$

By dropping the approximation parentheses and the order of approximation term, the final form of the particle approximation in a discrete form is

$$
f(x_i) = \sum_{j}^{N} \frac{m_j}{\rho_j} f(x_j) W_{ij},
$$

where $i$ and $j$ denote the interpolated particle and the neighbouring particles, respectively, and $W_{ij} = W(x_i - x_j, h)$. Particle $i$ is at the centre at the kernel function on a “gather” interpretation where the kernel acts as a weighting function to approximate the properties of the $i$ particle smoothed over its neighbours $j$ over a finite region of radius of support of $2h$. Therefore, particle $j$ contributes towards the particle properties of $i$ depending on the distance from the centre of the support over a compact region as depicted in Figure 3.2.
Figure 3.2. Support domain $\Omega$ of kernel $W$ when approximating particle $i$ located at the centre of the domain with a radius of $ah$ and particle $j$ located $x_{ij}$ distance away.

Particles move in space over time and thus a uniform particle distribution of initial particle spacing $\Delta x$ is not maintained with the exception of the initialisation. Since a particle is an interpolation point its volume is not related to the physical domain and therefore volume overlapping does not occur when particles move close to each other but overlapping kernels is an inherent part of the methodology. Particle volume and properties are related to the interpolation point and not to a physical particle. Figure 3.2 illustrates the particle approximation procedure. Also, particles are not restricted to a kernel support and are allowed to move in and out of any support of neighbouring particles by moving in space. Therefore SPH uses a local approximation of the quantities of the interpolating particles. Nevertheless, each support must have a good representation of neighbouring particles within its smoothing radius. Next, the discrete approximation of the derivative of function is presented.

3.4.2. Discrete approximation of the derivative of a function

By using the integral approximation of a spatial derivative of Equation (3.15) in the same basis as with the function approximation, the particle approximation of a spatial derivative can be written as

$$\nabla \cdot f(x_i) = -\sum_{j}^{N} \frac{m_j}{\rho_j} f(x_j) \cdot \nabla_j W_{ij},$$  \hspace{1cm} (3.19)

since
\[ \nabla_i W_{ij} = \frac{\mathbf{x}_i - \mathbf{x}_j}{r_{ij}} \frac{\partial W_{ij}}{\partial r_{ij}}, \quad (3.20) \]

where \( r_{ij} \) is the magnitude of the distance between particle \( i \) and \( j \). Using the even property of the smoothing function the kernel gradient can be written as \( \nabla_i W_{ij} = - \nabla_j W_{ij} \) with respect to the \( i^{th} \) particle or simply as

\[ \nabla \cdot f(\mathbf{x}_i) = \sum_j m_j \frac{f(\mathbf{x}_j)}{\rho_j} \cdot \nabla_i W_{ij}. \quad (3.21) \]

For simplicity onwards the subscript \( i \) will be dropped so as \( \nabla W_{ij} = \nabla_i W_{ij} \). This is the basic form of the spatial gradient in SPH formalism. However the gradient for a function \( f \) will give a non-zero approximation for a uniform \( f \), therefore other formulations can be derived for gradients. Using the identities

\[ \rho \nabla \cdot f(\mathbf{x}) = \nabla \cdot (\rho f(\mathbf{x})) - f(\mathbf{x}) \cdot \nabla \rho, \quad (3.22) \]

and

\[ \nabla \cdot f(\mathbf{x}) = \rho \nabla \left( \frac{f(\mathbf{x})}{\rho} \right) - \frac{1}{\rho} f(\mathbf{x}) \cdot \nabla \rho, \quad (3.23) \]

and by using Equation (3.19) and simplifying the notation further, these two identities yield

\[ \nabla \cdot f_i = \sum_j m_j \frac{f_j - f_i}{\rho_j} \cdot \nabla W_{ij}, \quad (3.24) \]

and

\[ \nabla \cdot f_i = \sum_j m_j \left( \frac{f_j}{\rho_j^2} + \frac{f_i}{\rho_i^2} \right) \cdot \nabla W_{ij}, \quad (3.25) \]

respectively and the \( \langle \cdot \rangle \) symbol denoting a vector dot product. The gradient of a constant function \( f \) in the case of Equation (3.24) is zero as would be expected. When force exchange between particles is considered requiring opposite and equal reaction that it has been proved to be essential to conserve angular momentum has to be satisfied [146], thus Equation (3.25) is favoured. Typically Equation (3.25) is used for the momentum equation whereas Equation (3.24) is used for the velocity gradients or the continuity equation. The above expressions hold for operators with higher dimensions.
From the derived equations it can be concluded that the particle approximation uses the integral representation to discretize the integral to discrete summations based on an arbitrary and finite set of particles. The finite support of the kernel function makes SPH a local interpolation scheme without the need of an interconnected mesh.

Furthermore, the particle approximation natively includes mass and density of particles in the summation due to the volume discretization. This is a key aspect of SPH for hydrodynamic problems since these two quantities are primitive field functions of fluid flows. Moreover, different discrete formulations can be derived to suit the function such as the variations of a gradient function or force exchange between particles [123]. Next, some fundamental properties with kernel examples and some arising issues are discussed.

3.5. Smoothing kernel

3.5.1. Fundamental properties of a smoothing kernel

SPH is based on an integral interpolation that depends on an effective way of performing the discrete approximation by a set of arbitrary scattered computational points without predefined connectivity between the points.

This is achieved by using a smoothing kernel that determines the pattern, the consistency and thus accuracy and the size of the support for the approximation. Equation (3.2) uses the Dirac delta function in the integral representation to recover a function $f$ exactly. Unfortunately, the support of the delta function tends to zero as $f$ is recovered. Thus, in a discrete finite domain the number of neighbouring particles would be zero. Therefore, a smoothing function with a finite support should be used that satisfies the delta function

$$\lim_{h \to 0} W(x-x',h) = \delta(x-x'). \quad (3.26)$$

In addition, a smoothing kernel should have a compact support with influence of $ah$ such as

$$W(x-x',h) = 0 \quad |x-x'| > ah, \quad (3.27)$$

that has already been used in the derivation of the integral representation. Outside of the support radius the kernel is zero. Some kernels such as the Gaussian kernel have infinite support but are not favoured in SPH due to $N^2$ interactions deeming such kernels unpopular due to the computational cost.

For a finite support, the kernel should be normalised over its support recovering unity so that
\[ \int_{\Omega} W(x - x', h) dx' = 1. \]  

This property is significant to recover zeroth-order polynomial consistency. A more detailed analysis on integral and particle consistency is given in later Sections 3.5.3 and 8 where some SPH approximation and boundary kernel truncation issues are discussed respectively.

Moreover, the kernel should be positive within the support domain with

\[ W(x - x', h) > 0, \]

when no kernel correction is used. This property is not necessary for convergence but avoids unphysical negative values of physical positive properties such as density, viscosity, mass, etc.

Furthermore, a kernel should decrease monotonically with distance from the centre to the edge of the support. Therefore particles closer to the kernel centre contribute more to the interpolated particle.

In addition to the above, it has already been stated in earlier Sections that a smoothing kernel should be an even function. The even property was demonstrated in the mathematical formalism, but in a discrete physical sense it implies that particles with the same distance and different positions have equal effect.

Finally, a kernel and its derivatives should be smooth avoiding slope discontinuities yielding better results with non-uniform particle distribution [172].

These seven properties are of essential significance for the integral representation and particle approximation. As with any discretization method, SPH should represent as closely as possible a function \( f \) leading to \( n \)-th order consistency. Violation of these “Axioms” results in reduction of the approximation order of accuracy and thus its consistency. Next some kernel examples are listed.

**3.5.2. Kernel examples**

A variety of different kernels have been used in SPH but in general kernels follow the following format

\[ W(R, h) = a_d f(R), \]

where \( a_d \) is the normalisation constant and \( R = |x - x'| / h \). The normalisation constant is usually defined as \( a_d = c / h^d \) where \( c \) is a constant and the superscript \( d \) refers to the number
of dimensions. Kernels mainly differ in shape and order but in general they follow the bell shaped Gaussian kernel first used by Gingold and Monaghan [70] in astrophysical simulations that is an even and monotonically decreasing function. Also, the Gaussian kernel is sufficiently smooth even for high orders of derivatives reducing numerical errors and tensile instability issues. However, the Gaussian kernel support theoretically extends to infinity; in a computational simulation as the kernel tends to zero it is considered compact but the large support deems the kernel cost prohibitive. The Gaussian kernel function reads

$$W(R,h) = a_d e^{-R^2},$$  \hspace{1cm} (3.31)

where \(a_d\) is \(1/\pi^{1/2} h, 1/\pi h^2\) and \(1/\pi^{3/2} h^3\) in one, two and three dimensions respectively.

Other popular kernels are based on B-splines which are piece-wise functions, such as the popular cubic spline [155] and the 5th-order quintic spline that uses a \(3h\) support [157]. A more extensive discussion on the B-splines kernels is given in [123] but it should be noted that the cubic-spline was not used in this study due to a linear piecewise function (i.e. non smooth) and larger computational cost.

In this work, the Wendland kernel has been used. It is a fifth-order kernel with compact support of \(2h\) avoiding the \(3h\) support of the quintic spline. Furthermore, it is not a piecewise kernel such as the B-splines thus reducing the computational cost and producing smoother derivatives. The Wendland kernel [210] reads

$$W(R,h) = a_d \left(1 - \frac{R}{2}\right)^4 (2R + 1),$$  \hspace{1cm} (3.32)

where the normalisation constant \(a_d\) is \(3/4h, 7/4h\pi^2\) and \(7/8h\pi^3\) in 1-D, 2-D and 3-D space, respectively. A comparison of the Gaussian and Wendland kernel and their gradients is shown in Figure 3.3 and Figure 3.4 respectively. The stability and accuracy of the scheme depends greatly on the kernel properties and the fundamental “Axioms” of the SPH scheme outlined in Section 3.5.1. Therefore, these fundamental properties ensure reproducibility of the SPH kernel approximation leading to a consistency of the some order. Next, some numerical issues arising from the last point are discussed.
Figure 3.3. The Gaussian and Wendland kernels for a 1-D space.

Figure 3.4. The Gaussian and Wendland first derivative for a 1-D space.
3.5.3. Numerical issues

Similar to any discretization method, in SPH the solution of a problem must approach the exact solution as $h \rightarrow 0$ and thus reproducing the Dirac delta function. We define the order of consistency as the order of which an approximation can reproduce a polynomial up to its $n$-th order exactly. This polynomial reproducibility would depend in the continuum formalism on the integral representation and the smoothing function itself. In a discrete domain, consistency is dependent upon the particle approximation [11].

For SPH to be zeroth-order consistent, a zero-order polynomial must be reproduced by the integral representation exactly thus,

$$c = \int_{\Omega} c W(x - x', h) dx',$$

(3.33)

where $c$ is a constant. Equation (3.33) is satisfied only when the kernel is normalised and the unity condition is satisfied. Therefore, for a kernel with compact support and normalised kernel the integral representation is said to have zeroth-order consistency.

First-order consistency is also possible with the integral representation, let $f(x) = ax + b$, thus

$$ax + b = \int_{\Omega} (ax' + b) W(x - x', h) dx',$$

(3.34)

or since zeroth consistency is met

$$x = \int_{\Omega} (x') W(x - x', h) dx'.$$

(3.35)

By using Equation (3.33)

$$x = \int_{\Omega} x W(x - x', h) dx',$$

(3.36)

and by subtracting the last two equations

$$0 = \int_{\Omega} (x - x') W(x - x', h) dx'.$$

(3.37)

The latter equation satisfies the linear consistency in SPH. Higher order consistency is possible but special partially negative kernel functions must be used which may result in unphysical domain properties such as density, viscosity, etc. More details on the consistency of the integral representation can be found in [127] and [126]. For that reason, SPH is said to
be first-order consistent in a continuum only when kernel truncation does not occur for instance around solid boundaries (see Section 8).

Figure 3.5. Particle approximation with (a) a uniform stencil, (b) non-uniform stencil and (c) kernel truncation due to boundary wall.

On the other hand, first-order consistency is not always satisfied for the particle approximation. For zeroth- and first-order consistency to be satisfied in a discrete formalism Equations (3.33) and (3.37) can be rewritten as

\[ 1 = \sum_{j}^{N} W(x - x', h) V_j, \]  

(3.38)

and

\[ 0 = \sum_{j}^{N} (x - x') W(x - x', h) V_j. \]  

(3.39)
For a regular (and uniform) particle distribution within the kernel the latter equations are satisfied and the method is said to be second-order. When particles are distributed irregularly inside the support the symmetry property does not apply leading to values less than unity in Equation (3.38) and a non-zero Equation (3.39). Similarly at a boundary the truncation of the kernel violates not only the symmetry but also the unity condition of the kernel. An illustration of the aforementioned inconsistencies is shown in Figure 3.5. To improve the kernel truncation by a wall boundary a new novel wall boundary condition has been developed and is being presented in Chapter 8 which approximately maintains zeroth and first-order consistency at the boundary.

3.6. Partial conclusions

This Chapter has presented the integral representation method of SPH which uses the convolution product of the function with a smoothing kernel to approximate a property of the domain. In discrete form it uses a summation of neighbouring interpolation points, called particles in the SPH formalism, to discretize the continuum. The smoothing kernel is an essential part of the method that needs to satisfy the “Axiom” conditions of SPH to maintain second-order accuracy in integral and discrete form. In discrete form strict first-order consistency can only be approximated maintained due to the disorder of the particles within the support or due to kernel truncation form solid boundaries. Nevertheless, the Lagrangian and local interpolation properties deem SPH a very versatile discretization scheme. The next Chapter presents the application of the SPH methodology to the governing equations for fluid mechanics.
4. Fluid dynamics and SPH discretization

4.1. Introduction

In this work, the Lagrangian form of the Navier Stokes equations are discretized using the SPH scheme to approximate the multi-phase applications of this thesis. The continuity and momentum equations with the addition of an equation of state are solved by the weakly compressible SPH (WCSPH) scheme. The multi-phase description is assumed to be energy independent and therefore, the energy equation is omitted. In addition, other closure models are presented herein.

It can be shown that a function $f$ can be written as

$$ f_L(x_i, t) = f_E(x_i, t) = f_E(r_i(x_i, t), t), $$

where $L$ denotes Lagrangian and $E$ Eulerian coordinates. Now, the rate of change of $f$ at the particle can be written as

$$ \frac{df}{dt} = \frac{\partial f_E}{\partial x_i} \frac{\partial x_i}{\partial t} + \frac{\partial f_E}{\partial t} \frac{\partial t}{\partial t} + u_i \frac{\partial f_E}{\partial x_i}, $$

or in a general form

$$ \frac{d}{dt} = \frac{\partial}{\partial t} + u_i \cdot \nabla. $$

That implies that the material derivate can be expressed as the local rate of change. The above is the relation between the Eulerian and Lagrangian formalism that can be used to describe convection terms in the Lagrangian description. Next, the governing equations and closure models are presented for a WCSPH scheme.

4.2. Conservation of mass

The evaluation of the density of a particle may be derived in a natural SPH formalism since a local interpolation of the density can be written in terms of Equation (3.18) as
\[ \rho_i = \sum_{j}^{N} m_j W_{ij}. \]  

(4.4)

Also, the continuity equation can be used in Lagrangian formalism with the form of

\[
\frac{d\rho}{dt} + \rho \nabla \cdot \mathbf{u} = 0. 
\]

(4.5)

By using the identity of Equation (3.22) the discretized form of the continuity equation is

\[
\frac{d\rho_i}{dt} = \mathbf{u}_i \sum_j^N m_j \nabla W_{ij} - \sum_j^N m_j \mathbf{u}_j \nabla W_{ij},
\]

(4.6)

or simply

\[
\frac{d\rho_i}{dt} = \sum_j^N m_j (\mathbf{u}_i - \mathbf{u}_j) \cdot \nabla W_{ij}. \]

(4.7)

The equivalent result will be deduced if the derivative approximation is applied to Equation (4.5) and by using the gradient in identity of Equation (3.39), constructing the symmetry formulation of Equation (4.7). Note, that by applying directly the gradient approximation on a constant gradient would not result to a zero value hence, the symmetric formulation is preferred.

Both formulations can be used to evaluate the density of a particle for an infinite and unbounded domain without differences between Equations (4.4) and (4.7). In a bounded domain, Equations (4.4) and (4.7) conserve the global and local mass of the system with

\[
\frac{dm}{dt} = 0. 
\]

(4.8)

However, with cases such as free-surface and interfacial flows Equation (4.4) lacks support at the edge which truncates the kernel under predicting the density in the vicinity of the free-surface [149]. In addition, by using the gradient of the kernel only the neighbouring particles are included in the summation avoiding spurious values from self-(particle)-contribution and is in line with the formulation of the momentum equation and the time integration scheme. Nevertheless, both formulations have been used in SPH [207]. In this work since interfacial and free-surface flows are important Equation (4.7) has been used.
Conservation of momentum

Newton’s second law for a particle $i$ can be written as

$$m_i \mathbf{a}_i = \mathbf{T}_i + \mathbf{S}_i,$$  (4.9)

where $m_i$ is the mass, $\mathbf{a}_i$ is the acceleration, $\mathbf{T}_i$ are the internal forces and $\mathbf{S}_i$ are the external forces acting on the particle $i$ such as gravity and other source terms. The Cauchy relation can be used to express the internal forces for a weakly compressible flow, namely the total stress tensor $\sigma$ made up from the isotropic pressure $p$ and the viscous stresses $\tau$ for particle $i$

$$\sigma_i = -p_i \mathbf{I} + \tau_i.$$  (4.10)

Dropping the $i$ subscript for convenience, the shear or viscous stresses are a linear function of the strain rate tensor for an isotropic fluid with

$$\tau = 2\mu \dot{\varepsilon},$$  (4.11)

where $\mu$ the dynamic viscosity and the deviatoric strain rate tensor is

$$\dot{\varepsilon} = \mathbf{D} - \frac{1}{3} \mathbf{D} \mathbf{I},$$  (4.12)

for a compressible fluid. The rate of deformation of the fluid depends on the velocity gradients

$$\mathbf{D} = \frac{1}{2} [\nabla \mathbf{u} + \nabla^T \mathbf{u}].$$  (4.13)

Thus, the momentum equation can be written as

$$\frac{d\mathbf{u}_j}{dt} = -\frac{1}{\rho_j} \frac{\partial \sigma_j}{\partial \mathbf{x}} + \mathbf{g},$$  (4.14)

where $\mathbf{g}$ is the gravity. Applying the SPH approximation using the identity of Equation (3.23) the momentum equation in SPH formalism can be written as

$$\frac{d\mathbf{u}_j}{dt} = \sum_{j}^{N} V_j \frac{\sigma_j}{\rho_j} \nabla W_j + \frac{\sigma_j}{\rho_j} \sum_{j}^{N} V_j \rho_j \nabla W_j + \mathbf{g},$$  (4.15)

or simply

$$\frac{d\mathbf{u}_j}{dt} = \sum_{j}^{N} m_j \left( \frac{\sigma_j}{\rho_j^2} + \frac{\sigma_j}{\rho_i^2} \right) \nabla W_j + \mathbf{g}.$$  (4.16)

A different approximation can be derived by using Equation (3.19) and
\[ 0 = \frac{\sigma_{i}}{\rho_{i}} \sum_{j}^{N} m_{j} \left( \frac{\sigma_{j}}{\rho_{j}} \right) \nabla W_{ij}, \]  
\quad (4.17)

yielding a slightly different form of the momentum equation

\[ \frac{du_{i}}{dt} = \sum_{j}^{N} m_{j} \left( \frac{\sigma_{i} + \sigma_{j}}{\rho_{j}} \right) \nabla W_{ij} + g. \]  
\quad (4.18)

Both formulations are equivalent and conserve linear and angular momentum by equal force exchange between particles \( i \) and \( j \) in the fluid (for more details see [146]). Equation (4.16) uses the square of the density at the denominator that may lead to inconsistent force exchange when a large density ratio between two fluids exits. Hence, Equation (4.18) is appropriate for large density variations between phases i.e. multi-phase flows and is used throughout this work. Further details of the discretization of the viscous forces are discussed below.

### 4.4. Pressure evaluation

The early development and application of SPH to gas dynamics in astrophysical problems [133] naturally employed the ideal law of gasses

\[ p = \rho R_{c} T, \]  
\quad (4.19)

where \( R_{c} \) is the gas constant and \( T \) is the temperature. Note that, either the energy equation is solved with this state equation, or, temperature is assumed constant for very small temporal isothermal pressure increases.

For liquids such as water, compressibility is minimal and present only at very large pressures. Hence, it is generally accepted that liquids can be modelled as incompressible fluids using an equation of state which accurately describes sound wave propagation in fluids [165]. Initially Batchelor [10] used such an equation of state that was recently modified by Monaghan [149] in the form of

\[ p = B \left( \frac{\rho}{\rho_{0}} \right)^{\gamma} - 1, \]  
\quad (4.20)

where \( B \) is a constant and \( \gamma \) the polytropic with values between 1 to 7 [94]. The choice of a large \( \gamma \), 7 in this work, tends to decrease tensile instability since the pressure increases significantly as particles approach each other as shown in numerical experiment in [95].
Equation (4.20) is often referred to as the Tait’s equation of state (EOS). The rational of the Tait’s EOS may be explained by using the relation of pressure-density for an isotropic ideal gas

\[
\frac{p}{p_0} = \left(\frac{\rho}{\rho_0}\right)^\gamma,
\]  

(4.21)

where the subscript 0 denotes the reference values. At the free surface in steady state, the pressure should equate to zero and the density is said to be the reference density of the liquid. The reference pressure of Equation (4.21) should be sufficiently large to relate the speed of sound to the density variation or

\[
C_{s0} = \sqrt{\frac{\partial p}{\partial \rho}}.
\]  

(4.22)

By differentiating Equation (4.21) it can be shown that

\[
p_0 = B = \frac{C_{s0}^2 \rho_0}{\gamma},
\]  

(4.23)

at the free-surface. Hence, the compressibility of the fluid is proportional to the speed of sound of the medium. The addition of minus unity of Equation (4.20) ensures automatically zero pressure at the free-surface as the density becomes equal to the reference density.

Small density variations with respect to the reference density, correspond to large pressure variations for large values of \(\gamma\). Bearing in mind the large speeds of sounds for a nearly incompressible fluid, the corresponding time step would become impractically small and tend to zero as \(C_{s0} \to \infty\). A justification of the latter expression is given in Section 4.8.1 following a Courant-Friedrich-Levy (CFL) condition rationale.

To reduce the physical speed of sound Monaghan [149] introduced a numerical speed of sound leading to an artificial compressibility with the numerical speed of sound related to the Mach number as \(M_a \approx u_{\text{max}} / C_{s0} \approx 0.1\) where \(u_{\text{max}}\) is the maximum velocity magnitude in the domain leading to a weakly compressible SPH. The numerical speed of sound is then

\[
C_{s0} = 10 u_{\text{max}}.
\]  

(4.24)

It is common to use much larger values of the coefficient such as 15 or 20 increasing the numerical speed of sound either due to closure models such as boundary conditions or due to
the physics of the problem to capture acoustic wave propagation in a numerical sense (see Section 4.8.1).

In free-surface flows and general hydraulic applications the maximum velocity can be related to the maximum potential energy of the system thus

\[ u_{\text{max}} = \sqrt{gH}, \]  

(4.25)

where \( H \) is the maximum height of the liquid since it would relate to the maximum kinematic energy of a closed system. Due to the polytropic index of 7 in the density ratio and the artificially compressible approach, small density variations may lead to large pressure fluctuations resulting in an erroneous pressure field. These spurious pressure fluctuations are addressed in the next Section.

Other possible equations of state include the a simplified version of the Tait’s equation of state, usually referred to as Morris equation of state [157] that reads

\[ p = C_{s0}^2 (\rho - \rho_0). \]  

(4.26)

Equation (4.26) is not stiff as the Tait’s equation of state with a density ratio of 3% from its reference density [157]. However, for moderate to high Reynolds number the pressure gradients obtained by the use of the Morris equation of state tend to be noisy and the compressibility increases sufficiently.

Another approach is to solve the Poisson equation in an incompressible SPH formalism (ISPH). A common ISPH Poisson equation solver uses the equation

\[ \nabla \cdot \left( \frac{1}{\rho} \nabla p \right) = \frac{1}{\Delta t} \nabla \cdot \mathbf{u}^*, \]  

(4.27)

where \( \mathbf{u}^* \) is the intermediate velocity of the projection method of Chorin [40] as used by Cummins and Rudman [48] a in divergence-free velocity field. This approach requires an implicit solver to solve the Poisson equation with the advantage of larger time steps but higher computational cost in the implementation and other numerical problems such as particle instabilities. Herein, only WCSPH is considered. For further information the reader is directed to the work by [48], [212] and more recently [117].
4.5. Density filtering

The WCPSH scheme has the advantage of an explicit formulation with a numerical speed of sound lower than the physical making the scheme versatile. Nevertheless, the spurious pressure field resulting from the high polytropic index in the EOS introduces errors in the kinematics of the momentum equation and the local approximation of a particle. To overcome this problem a zeroth- [183] and first-order[43] density filter have been used widely in SPH codes [44, 72, 189]. Additionally, models such as δ-SPH have been developed based on a thermodynamic principle of thermal dissipation [59, 145]. These approaches are introduced below.

Shepard filter

The Shepard filter is a zeroth-order correction reproducing only a zeroth-order polynomial by reinitialising the density every $N$ time steps, usually every 20–40 time steps. To achieve this the kernel is corrected using the a unit summation of the local support on the denominator over the kernel itself as

$$
\tilde{W}_i = \frac{W_{ij}}{\sum_j m_j W_{ij}}.
$$

(4.28)

resulting in a kernel correction. The corrected density is then recalculated as an SPH approximation using the corrected kernel

$$
\rho_i^{\text{new}} = \sum_j m_j \tilde{W}_{ij}.
$$

(4.29)

Note that, the formulation of Equation (4.29) is identical to the formulation of the density of Equation (4.4) which performs poorly on the free-surface but in this case it includes a corrected kernel for the truncated region. Even if the Shepard filter is only first-order accurate it is widely used in SPH as a result of the simplicity and low computational cost.

Moving Least Squares (MLS)

Colagrossi and Landrini [43] applied the popular MLS scheme to SPH achieving first-order consistency and thus reproducing the linear variation of the density across the domain. The filter is applied every 20–40 time steps similar to the Shepard filter. The MLS density filtering requires matrix inversion (4x4 in 3-D) for each particle that may prove
computationally expensive for large number of particles. Even it is first-order consistent it has not been used in this work as the formulation is not well suited to GPU programming.

δ-SPH

The δ-SPH implementation [145] is not a density filtering algorithm as with the Shepard and MLS filter but a diffusion term included in the density equation. It is derived from a thermodynamic point of view accounting for the thermal effects of a weakly compressible fluid thus diffusing the density by

\[ D_{\delta-SPH} = \delta_d h C_{s0} \sum_j^N \frac{m_j}{\rho_j} \psi_{ij} \nabla \rho_{ij}, \]  

(4.30)

with

\[ \psi_{ij} = 2(\rho_j - \rho_i) \frac{x_{ij}}{|x_{ij}|^2 + 0.1h^2}, \]  

(4.31)

where \( \delta_d \) is a tuning parameter usually set to 0.1 [139]. The diffusion term has similarities with the artificial viscosity of Monaghan [150]. The δ-SPH has been used extensively in this work due to its simplicity, low computational cost and suitability to GPU programming.

4.6. Viscous models

Artificial viscosity

The viscous forces in SPH have been traditionally modelled using the artificial viscosity [150] through an artificial pressure term

\[ \Pi_v = \begin{cases} -a_v \bar{c}_{ij} u_{ij} + \beta_v \bar{\rho}_{ij}^2, & \text{if } u_{ij} \cdot x_{ij} < 0, \\ 0, & \text{if } u_{ij} \cdot x_{ij} \geq 0 \end{cases}, \]  

(4.32)

with

\[ \mu_{ij} = \frac{h u_{ij} \cdot x_{ij}}{|x_{ij}|^2 + \eta^2}, \]  

(4.33)

where \( a_v \) and \( \beta_v \) are constants depending on the case, \( \bar{c}_{ij} \) and \( \bar{\rho}_{ij} \) is the average numerical speed of sound and average density, respectively, of the interpolated particle and the neighbours respectively and \( \eta^2 = 0.01h^2 \) is inserted to avoid a numerical singularity when
$|x_{ij}| \rightarrow 0$. From equation (4.33) the artificial viscosity associated with the $a_v$ parameter will only produce bulk forces in compression and will tend to zero as $h \rightarrow 0$. The parameter $\beta_v$ is intended to suppress particle interpenetration and clumping at high Mach numbers and allow shocks to be simulated. The constants can be tuned to a specific Reynolds number by a kinematic viscosity $[152]$ that reads

$$v = \frac{1}{8} a_v h \bar{\sigma}_0.$$

The artificial viscosity formulation in SPH is based on the von Neumann-Richtmeyer artificial viscosity $[208]$ initially developed for high Mach number problems such as shock waves in mesh based methods accounting for the dissipation of the kinetic energy to heat energy removing numerical oscillations in simulation. However, the empirical nature of the artificial viscosity prompts for a more rigorous treatment of the viscous forces.

**Laminar viscosity**

Morris $[157]$ avoid the use of the second derivative of the kernel by using a mixture of a standard SPH first derivative interpolation and a finite differences approximation for the velocity gradients to formulate a laminar viscosity expression. The viscous term can be expressed in SPH formalism as

$$\frac{1}{\rho} \frac{\partial \boldsymbol{\tau}_i}{\partial \mathbf{x}_i} = \sum_j m_j \left( \frac{\boldsymbol{\tau}_j + \boldsymbol{\tau}_i}{\rho_j \rho_i} \right) \nabla W_{ij}.$$  \hspace{1cm} (4.35)

The viscous stresses of Equation (4.11) can be computed by using the velocity gradients using a finite difference approximation as

$$\frac{\partial \mathbf{u}_i}{\partial \mathbf{x}_i} = \frac{\mathbf{u}_i - \mathbf{u}_j}{r_{ij}} \cdot \frac{\mathbf{x}_i - \mathbf{x}_j}{r_{ij}},$$  \hspace{1cm} (4.36)

where $r_{ij}$ is the magnitude of the distance between the two particles. By combining Equation (4.35), (4.11) and (4.36) with the assumption of incompressibility for a laminar flow, the laminar viscosity formulation can be expressed as

$$v \Delta \mathbf{u}_i = \sum_j m_j \left( \mu_i + \mu_j \right) \mathbf{u}_j \cdot \frac{\mathbf{x}_{ij}}{|\mathbf{x}_{ij}|^2 + \eta^2} \nabla W_{ij},$$  \hspace{1cm} (4.37)

where $\eta^2 = 0.01 h^2$ is inserted to avoid a numerical singularity and $\Delta$ denotes the Laplacian. The expression of Morris $[157]$ uses the tangential forces to express the viscous forces in the momentum equation which is physically meaningful. In addition, linear momentum is
conserved but not angular momentum [74]. However, the assumption of incompressibility deems the shear stress interpolation applicable only to laminar flows. The Morris formulation [157] uses the physical dynamic viscosity and is preferred over the empirical model of Monaghan [150]. Similar formulations have been developed in SPH such as the Monaghan-Cleary-Gingold [148] laminar viscosity model. A comprehensive review of the viscosity formulations can be found in [74] and [207].

**Arbitrary formulation**

Since liquid-soil scour and resuspension is dominated by the viscous characteristics at the interface, a thorough viscous formulation is necessary. In this thesis an approach used initially in astrophysics by Speith et al. [188] is employed. As with the laminar formulation of Morris [157], the viscous term can be expressed in SPH formalism as

\[ \frac{1}{\rho} \frac{\partial \tau_{ij}}{\partial x_j} = \sum_j m_j \left( \frac{\tau_{ij}}{\rho_j} + \frac{\tau_{ji}}{\rho_i} \right) \nabla W_{ij}. \]  

(4.38)

Previously the velocity derivative was calculated through a finite difference approximation with the assumption of incompressibility. Herein, the velocity derivatives use a symmetric SPH approximation in the form of

\[ \frac{\hat{u}_i}{\partial x_i} = \sum_j m_j \left( \frac{u_j - u_i}{\rho_j} \right) \nabla W_{ij}. \]  

(4.39)

used to calculate the strain rate tensor of Equation (4.12). Hence, the viscous stresses of Equation (4.11) can be obtained. Finally, the viscous stresses are substituted to Equation (4.38) to approximate the viscous part of the momentum equation. This formulation conserves linear and angular momentum and is valid for higher Reynolds numbers and turbulence subject to a turbulence closure model that is described next. In addition, it allows for different constitutive equations to be modelled with little effort by replacing the Newtonian constitutive equation.

However this rigorous formulation tends to be expensive since the velocity gradients needs to be known *a priori*. In a general methodology, the velocity gradient and viscous stresses can be calculated initially and saved to the memory. After a second particle sweep can be used to obtain the viscous part of the momentum formulation that results to $2N\log N$ interactions.
4.7. Turbulence modelling

Turbulence modelling in SPH has received significant attention the last few years since the nature of flows that SPH commonly model are violent non-linear flows where turbulence is a dominant feature at high Reynolds numbers.

One approach to resolve turbulence is by using Direct Numerical Simulation (DNS) where the Navier Stokes equations are solved without any additional modelling techniques. In order to capture the turbulent structures generated in the flow field one must resolve to length scales of the smallest eddy formation. However, resolving the Kolmogorov scales as the Reynolds number increases is prohibited by the huge computational resources. DNS simulations using grid-based methods range from Re = 5000 [101] to around 50 000 since the memory requirements scale as Re^3 and computational cost as Re^4. The largest DNS simulation to date is by Lee et al. [110] of wall bounded turbulent flow with Re = 5000. SPH DNS simulations include the work by Robinson et al. [175] in 2-D and lately Mayrhofer et al. [141] in 3-D.

Large Eddy Simulations (LES) as the name suggested, resolves the large eddies of the flow and models the small structures with a sub-grid scheme. The resolution of the large eddies only, allows for length scales much larger than the DNS simulation and therefore coarser grid (or particle) resolution can be used. However, optimal meshing in LES can be challenging when the spatial filtering is applied for under resolved length-scales since the turbulent scales are not known in advance [178]. In SPH, LES modelling and more specifically the Smagorinsky model has been used in SPH by many researchers in 2-D [49, 94, 143] and 3-D [141].

The Reynolds averaged Navier Stokes (RANS) equations have been used successfully for modelling turbulent flows by applying an average over an infinite time. The technique yields information of the mean and fluctuating flow quantities by relating the Reynolds stresses to the mean strain rate using a turbulent viscosity [169]. Several one and two or more equation models exist such as the Prandtl mixing length model, $k - \varepsilon$, $k - \omega$ and the Reynolds-stress model (see Pope [169]). In SPH, the $k - \varepsilon$ model has been popular due to its low computational cost and wide applicability in comparison with the $k - \omega$ or other more complicated models [207]. The RANS models are popular in fluid dynamics in grid- or mesh-based methods due to the low resources they require using a statistical approach [169].
Herein, a Large Eddy Simulation (LES) model has been used to model the turbulent characteristics of a multi-phase flow. The LES model is a standard Smagorinsky algebraic eddy viscosity model as described by Dalrymple and Rogers [49] in a WCSPH formalism for Newtonian fluids.

In combination with the arbitrary viscous formulation the turbulent viscosity is accounted as

\[ \mu = \mu_p + \mu_t, \]  

where \( \mu_p \) denotes the molecular dynamic viscosity and \( \mu_t \) the eddy turbulent viscosity. The turbulent viscosity is calculated using a low pass scale filter length usually equal to the smoothing length \( h \) in SPH and the Smagorinsky constant \( C_s \) according to

\[ \mu_t = \rho (C_s h)^2 \frac{1}{2} \sqrt{\frac{1}{2}I_D}. \]  

A typical formulation for the Smagorinsky constant is used in the form of

\[ C_s = \frac{1}{\pi} \left( \frac{3}{2} C_k \right)^{-3/4}, \]  

where \( C_k \) is the Kolmogorov constant typically equal to 1.5 [49]. The turbulent viscosity formulation is only applied to the liquid phase and the sediment phase with a concentration soil fraction of smaller than 0.3 since the shear layer is mainly laminar.

However, it should be pointed out that the turbulence effects in a liquid-sediment fluvial region (i.e. the interface) and during the resuspension of the sediment in the liquid can influence the flow field and resuspension considerably. Using a LES model in SPH and specifically near the interface or boundary, the discretization is likely to be coarse and with grid size larger than the eddy length scales as demonstrated by Mayrhofer et al. [141]. The large computational cost for fine resolutions is one of the major issues with SPH when LES modelling is employed.

This study has not consider if the turbulence has been resolved sufficiently, however it is an important aspect of the liquid-sediment flow field and should be investigated further.
4.8. Numerical implementation

4.8.1. Temporal integration

The time integration schemes used in WCSPH are explicit of at least second-order accuracy in time since SPH is a meshless second-order accurate method spatially with moving interpolation points. Let the momentum, continuity and spatial equation for a particle $i$ be

\[
\frac{du_i}{dt} = F_i, \\
\frac{d\rho_i}{dt} = \xi_i, \\
\frac{dx_i}{dt} = u_i,
\]

respectively. The Lagrangian derivative of Equations (4.43) can be integrated by using 2nd order symplectic schemes or higher order explicit schemes.

Predictor-corrector scheme

The predictor-corrector scheme [205] is a second-order explicit integrator scheme. It is a two step algorithm where forces are predicted in the first step using an Euler forward step at half time and then the corresponding values are corrected using forces at half time step, finally the integration values are calculated at the end of the step. The scheme can be written as

- **Predict**

\[
\begin{align*}
\mathbf{u}_{i,p}^{n+1/2} &= \mathbf{u}_i^n + \mathbf{F}_i^n \frac{\Delta t}{2} \\
\rho_{i,p}^{n+1/2} &= \rho_i^n + \xi_i^n \frac{\Delta t}{2}, \\
x_{i,p}^{n+1/2} &= x_i^n + u_i^n \frac{\Delta t}{2}
\end{align*}
\]

- **Correct**

\[
\begin{align*}
\mathbf{u}_{i,c}^{n+1/2} &= \mathbf{u}_i^n + \mathbf{F}_i^{n+1/2} \frac{\Delta t}{2} \\
\rho_{i,c}^{n+1/2} &= \rho_i^n + \xi_i^{n+1/2} \frac{\Delta t}{2}, \\
x_{i,c}^{n+1/2} &= x_i^n + u_i^{n+1/2} \frac{\Delta t}{2}
\end{align*}
\]

- **Final step**

\[
\begin{align*}
\mathbf{u}_i^{n+1} &= 2\mathbf{u}_{i,c}^{n+1/2} - \mathbf{u}_i^n, \\
\rho_i^{n+1} &= 2\rho_{i,c}^{n+1/2} - \rho_i^n, \\
x_i^{n+1} &= 2x_{i,c}^{n+1/2} - x_i^n
\end{align*}
\]
where \( n, n + 1/2 \) and \( n + 1 \) is the current, current plus half and next time step of the property. The current model has been used in this work. The predictor corrector tends to be numerically stable because of the predict-correct step it employs even for large time steps [205].

**Velocity Verlet scheme**

The velocity Verlet scheme [205] has been very popular with Molecular Dynamics which is explicit as well. It is also a two step algorithm but the second step is only applied every \( N \) steps (usually 40 time steps). Variables are calculated according to

\[
\begin{align*}
    u_i^{n+1} &= u_i^n + 2F_i^n \Delta t \\
    \rho_i^{n+1} &= \rho_i^n + 2\ddot{\xi}_i^n \Delta t \\
    x_i^{n+1} &= x_i^n + u_i^n \Delta t^2 / 2
\end{align*}
\]

(4.47)

Note, the different formulation of the position prediction. Since the equation are decoupled in the previous step, the following is applied every \( N \) steps to stop the integration drifting

\[
\begin{align*}
    u_i^{n+1} &= u_i^n + F_i^n \Delta t \\
    \rho_i^{n+1} &= \rho_i^n + \ddot{\xi}_i^n \Delta t \\
    x_i^{n+1} &= x_i^n + u_i^n \Delta t + u_i^n \Delta t^2 / 2
\end{align*}
\]

(4.48)

by using the current step \( n \) to compute the new values for all three equations and applying a Euler 1st order step in the velocity and density a second-order Euler step on the drifting position coupling the forces.

**Other time integration schemes**

Other second-order symplectic schemes include examples such as the leap-frog scheme [32] which are similar to the Verlet scheme. The position and velocity is decoupled; with the first leaping an entire time step and the velocity computed at every half time step or using a kick and drift step providing a staggered arrangement by using a central difference type scheme [32]. Physical quantities are usually evaluated every full step with the velocity doing a half backward Euler step for synchronising. The leap-frog scheme was initially investigated since it uses a solver only once making it computationally cheaper. However for marginally large time steps it may lead to numerical instabilities [32] and therefore the predictor-corrector was used instead.

Other higher order schemes include the Beeman and fourth-order Range-Kutta scheme from Molecular Dynamics. The schemes tend not to be used in SPH because of computational cost considerations.
### 4.8.2. Variable time step

The Courant-Friedrich-Levy condition (CFL) is imposed on the size of the time step to ensure that a signal travelling though the domain is resolved thus it is less or equal to the convection time over the characteristic length of the discretization scheme. In SPH the wave propagation can be written using the numerical speed of sound and the smoothing length as

\[
\Delta t_{C,\alpha} = \frac{h}{C_{\alpha} + \max_i \left| \frac{h u_i \cdot x_i}{|x_i|^2} \right|},
\]

where an equivalent expression for resolving sufficiently the forces in the domain can be written as [146]

\[
\Delta t_F = \min_i \frac{h}{\sqrt{|f_i|}},
\]

where \( f_i \) is the force per unit mass of particle \( i \). Due to potentially large viscous forces in a multi-phase simulation the time step is restricted by the viscous forces

\[
\Delta t_\mu = \frac{1}{2} \frac{\rho h^2}{\mu},
\]

based on the characteristic length and dynamic viscosity. Morris [157] argued that the viscous time restriction is necessary as \( h \) is decreasing and the Reynolds number is increasing.

Finally, the variable time step [154] is calculated as

\[
\Delta t = Co \min(\Delta t_{C,\alpha}, \Delta t_F, \Delta t_\mu),
\]

where \( Co \) is the Courant number that usually is determined by numerical experiments [32]. In this work \( Co = 0.1 \) has been used.

All three time step restrictions have been applied in this work with the dominant \( \Delta t \) restriction that of the speed of sound primarily. The force time step restriction by numerical experiments has rarely be found to be the principal restriction for this work since the forces between particles are over resolved by the numerical speed of sound \( \Delta t \), thus, the wave propagation signal over resolves the force exchange within the domain. That may not be the case for fracture solid mechanics and violent impact flows or shockwaves [123]. Finally the viscous \( \Delta t \) only is only effective for very fine resolutions in large 2-D cases with large viscosity and generally is of the same magnitude as that due to the speed of sound.
4.8.3. Wall Boundary conditions

The wall boundary condition applied in this thesis is the Dynamic Boundary Conditions (DBCs) [45]. Boundary particles representing the wall are organised in a staggered arrangement as shown in Figure 4.1. Boundary particles satisfy the same equations as the fluid particles but their spatial position is fixed with the exemption of moving boundaries. When a fluid particle is approaching the wall boundary the density of the boundary particles increases according to the continuity equation of (4.7) resulting in a pressure increase in the momentum equation (4.16) and repulsion force to the fluid particle. The advantages of the DBC are the fairly easy computational implementation and the treatment of arbitrary complex geometries. However the wall treatment suffers from unphysical repulsion forces and particle penetration for moderate numerical speeds of sound.

![Figure 4.1 2-D sketch of the staggered particle arrangement of the boundary particles in DBC (black) and an approaching fluid particle (white).](image)

Wall boundary conditions have attracted great attention in SPH and are considered to be an unresolved research area. The DBCs have been used in DualSPHysics and this work due to the low computational cost and simplicity. In this work a more rigorous wall boundary condition has been developed and is presented in Chapter 8. However, the new wall boundary condition has not been used in the multi-phase results presented herein since it is still under development and it is being extended to 3-D with a GPU implementation.

4.8.4. Computational efficiency

SPH simulations are expensive in comparison with other mesh based methods (e.g. FVM and FEM) because of the large number of neighbouring particles within the kernel. Furthermore since particles move in space, particles within a support are allowed to leave or enter. Therefore, identification of the neighbours (or support neighbours) is required though an efficient neighbour search algorithm. An all-pair search would result in $N^2$ interactions, which
is numerically prohibitive for large number of particles and is only applicable to academic demonstration purposes (see Figure 4.2).

There are three main neighbour search algorithms used currently in SPH, the linked-list [155], Velret list [55] and the tree-search algorithms [84]. The latter search algorithm, as the name suggests creates tree-like order of particle according to the particle position [84]. When the tree mapping has completed, the position of the nearest neighbouring particle is achieved with an order of $N \log(N)$ interactions. Mainly, the tree-search algorithm has been used for variable smoothing length astrophysical simulations [189].

For a constant smoothing length algorithm employed in a GPU implementation (see Section 6.4) the linked-list or Verlet list search algorithm is preferred. Following Monaghan and Latanzio [155] the computational domain is divided into square cells of size $ah$. The interpolated particle located in the cell only has to interact with the particles of the surrounding cells which are 3, 9 or 27 cells in 1-D, 2-D and 3-D respectively for a typical $a = 2$. This temporary mesh of cell size $ah$ reduces the particle interactions form $N^2$ to $N \log(N)$ similar to the tree-search algorithm (see Figure 4.2). For variable smoothing length, since $ah$ is not constant, the size of the cells will not be optimal for every particle reducing the algorithm efficiency. Figure 4.3 illustrates the linked-list algorithm for searching the nearest neighbouring particles in two-dimensional space.

Figure 4.2. Comparison of the $N^2$ all pair search to the $N \log(N)$ linked-list algorithm.
4.9. Partial conclusions

In this Chapter, the Navier Stokes equations in SPH formalism were presented with closure models completing the system of equations with the addition of the numerical implementation.

The Navier Stokes equations were formulated using a symmetric SPH approximation ensuring mass and momentum (linear and angular) conservation. The density was related to pressure through a weakly compressible approach using the Tait’s equation of state by using a numerical speed of sound lower than the physical speed of sound to increase the time step of the time integration. Density fluctuations arising from the large exponent polytropic index in the density ratio that lead to spurious pressure fluctuations are smoothed out by using a density filter with zeroth consistency or by a diffusive term in the continuity equation in this work. The viscous part of the Navier Stokes equations is approximated using a rigorous formulation derived directly from the governing equations that conserves linear and angular momentum and can be applied to higher Reynolds numbers by using a turbulence closure model such as the Smagorinsky model. Wall boundary conditions are applied by using the dynamic boundary conditions.
The mechanism of a variable time step has been demonstrated based on the speed of sound acoustic propagation, force exchange between particles and viscous forces through a CFL condition to ensure the numerical domain is resolved adequately. Finally, the neighbour list used in this work has been examined through a linked-list search algorithm reducing the search computational effort from $N^2$ to $N\log(N)$. In the following Chapter the multi-phase implementation is discussed.
Chapter 5

5. Multi-phase liquid-sediment SPH model

5.1. Introduction

This chapter presents the multi-phase model developed in this thesis for treating liquid – soil flows. These multi-phase flows are dominated by the phase characteristics and the interfacial boundary where the soil shear layer and suspension of the granulate sediment is taking place. Under rapid liquid flows the scour at the soil interface is dominated by the yield characteristics of the sediment creating a shear non-Newtonian layer of the saturated soil mixture with particle suspension in a form of bed load and/or suspended load.

Both phases are examined as a continuous material in a macroscopic approach which is well suited technique for the liquid phase medium. However, the soil phase consists of discrete granular material saturated with liquid. The saturated soil mixture has been traditionally modelled using a continuous material method in SPH (see Section 2.7) by using the mixture properties such as density, pressure, viscosity and Coulomb parameters. Herein, the soil mixture is represented by discrete SPH particles using the mixture physical properties. Hence, a single SPH particle may comprise of a number of fully saturated granulates depending on the soil characteristics such as porosity and concentration.

These subaqueous sediment scouring flows are induced by a rapid inflow creating a failure surface in the sediment phase. The sediment yielded surface is being determined using a Coulomb model, namely the Drucker-Prager yield criterion based on the Coulomb parameters of cohesion, angle of repose (internal friction angle) and the sediment skeleton pressure. The yielded surface under large relative motions between the phases produces a shear layer of suspended particles at the interface. This shear layer is directly affected from the dynamics of the liquid phase and the shear forces applied to the yielded surface. The large concentration of sediment in the shear layer resembles a non-Newtonian flow with Bingham flow characteristics. In this thesis the Herschel-Buckley-Papanastasiou [166] model is used to model the non-Newtonian behaviour of the shear layer. At the interface of the liquid – soil mixture, a bed load with a suspension mechanism is formed. The sediment suspension at the
interface of the yielded shear layer induced by the liquid is characterised by a suspension viscosity. Suspended particles in the fluid are modelled with a suspension viscosity based on the Vand experimental colloidal suspension equation [203] of sediment in a fluid using the suspension viscosity in the local interpolation instead of a mesoscopic approach resolving the motion and collision mechanism. This suspension viscosity mechanism is based on the volumetric fraction of the sediment in the liquid. The suspension layer is modelled using the Newtonian constitutive equation.

Other approaches have been developed for the suspension of the sediment by the fluid based on volumetric concentrations and colloidal frictional suspensions with most recently the water/soil-suspension layer of Ulrich et al. [195] that bears some similarities with the current model. However, in this work the shear layer characteristics involve a more comprehensive soil description. In this Chapter, a detailed description of the liquid and soil phase mathematical formulation with the addition of the sub-closure models is presented. Indicial notation is used to denote vectors and tensors written by rectangular Cartesian components using the Greek superscripts \(\alpha\) and \(\beta\) whereas \(i\) and \(j\) subscripts denote the interpolated particle and its neighbour, respectively.

### 5.2. Liquid model

#### 5.2.1. Newtonian viscous formulation

Following Stokes’ theorem a general fluid can be written using the thermodynamic pressure and the extra stress tensor in the form of

\[
\sigma^{\alpha\beta} = -p\delta^{\alpha\beta} + f(D^{\alpha\beta}),
\]  

(5.1)

based on the assumption that the difference between the stress in a deforming fluid and the static equilibrium stress is given by the function \(f\) determined by the rate of deformation. When the function \(f\) is linear for an isotropic material the fluid is called Newtonian and the constitutive equation can be written in the general form of

\[
\sigma^{\alpha\beta} = -p\delta^{\alpha\beta} + \lambda D^{\gamma\gamma} \delta^{\alpha\beta} + 2\mu_d D^{\alpha\beta},
\]  

(5.2)

where \(\lambda\) and \(\mu_d\) are constants. Equation (5.2) is the Navier–Poisson equation for a Newtonian fluid. Using Stokes condition assumption \(\lambda = -2/3 \mu_d\), Equation (5.2) takes the form of
\[
\sigma^{\alpha\beta} = -p\delta^{\alpha\beta} - \frac{2}{3}\mu_dD^{\alpha}\delta^{\alpha\beta} + 2\mu_dD^{\alpha\beta},
\]

(5.3)

where the constant \(\mu_d\) is the dynamic viscosity of the fluid. The above expression can be written in its final form in terms of the deviators as

\[
\sigma^{\alpha\beta} = -p\delta^{\alpha\beta} + 2\mu_d\varepsilon^{\alpha\beta},
\]

(5.4)

where \(\varepsilon^{\alpha\beta}\) denotes the strain rate tensor. Note that in incompressible flow \(\varepsilon^{\alpha\beta} = D^{\alpha\beta}\) since the \(D^{ij}\) is zero by the continuity equation. In the WCSPH formalism Equation (5.4) can be used to obtain the total stresses in the momentum equation, thus the strain rate tensor can be calculated for the velocity gradients as

\[
\varepsilon^{\alpha\beta} = \frac{1}{2}\left[ \sum_j^N \rho_j m_j u_{ij}^\alpha \frac{\partial W_{ij}}{\partial x_i^\alpha} + \sum_j^N \rho_j m_j u_{ij}^\beta \frac{\partial W_{ij}}{\partial x_i^\beta} \right] - \frac{1}{3}\left[ \sum_j^N \rho_j m_j u_{ij}^\gamma \frac{\partial W_{ij}}{\partial x_i^\gamma} \right] \delta^{\alpha\beta},
\]

(5.5)

using the deviatoric strain rate tensor. Therefore, the deviatoric (or viscous) stress tensor can be calculated from the Newtonian constitutive equation that relates the strain rates to the viscous stresses by

\[
\tau^{\alpha\beta} = 2\mu\varepsilon^{\alpha\beta}.
\]

(5.6)

The total viscosity \(\mu\) of Equation (5.6) represents the dynamic and eddy viscosity \(\mu_e\) obtained through the Smagorinsky algebraic eddy viscosity model of Section 4.7 by

\[
\mu = \mu_d + \mu_e.
\]

(5.7)

The pressure of Equation (5.3) is the mean pressure that equals to the thermodynamic pressure of the EOS only for incompressible flow since

\[
p = \bar{p} - \kappa D^{ij},
\]

(5.8)

where \(\bar{p}\) is the thermodynamic pressure and \(\kappa\) is the bulk viscosity of the fluid. The bulk viscosity is directly related to the Stokes condition which is the second condition for the mean pressure to equal the EOS pressure if \(\kappa = \lambda + 2 / 3\mu = 0\). The Stokes condition acts as the dissipation to the mean pressure due to the bulk viscosity.

### 5.2.2. \(\delta\)-SPH

In WCSPH, the mean pressure is calculated directly from the equation of state after a zeroth of first order filtering operation has been applied to the density field as explained in Section 4.5 applied every 20 – 30 time steps. In this thesis the \(\delta\)-SPH approach is used to account for
the bulk viscosity dissipation in the mean pressure by a dissipation term applied to the continuity equation as presented in Section 4.5 similar to the Stokes condition. However, the current δ-SPH formulation is based on an empirical artificial dissipation which is not related to the bulk viscosity in a similar manner to the artificial viscosity of Monaghan [150]. The final form of the momentum equation in SPH formalism for the Newtonian liquid takes the form of

\[
\frac{du_i^\alpha}{dt} = -\sum_j m_j \left( \frac{p_i + p_j}{\rho_i \rho_j} \right) \frac{\partial W_{ij}}{\partial x_i^\alpha} + \sum_j m_j \left( \frac{\tau_{ij}^{\alpha\beta} + \tau_{ij}^{\beta\alpha}}{\rho_i \rho_j} \right) \frac{\partial W_{ij}}{\partial x_i^\beta} + g_i^\alpha, \tag{5.9}
\]

where the first term on the right hand side of Equation (5.9) accounts for the thermodynamic pressure forces, the second for the viscous forces and the third term the gravitational forces.

The continuity equation in WCSPH does not guaranty a divergence-free velocity field and the equation takes the form of

\[
\frac{d\rho_i}{dt} = \sum_j m_j (u_i^\alpha - u_j^\alpha) \frac{\partial W_{ij}}{\partial x_i^\alpha} + D_{\delta-\text{SPH},i}, \tag{5.10}
\]

where the first term on the right hand side of Equation (5.10) is the velocity divergence and the second term the δ-SPH dissipation to the continuity equation.

### 5.2.3. Particle shifting

While particles’ general variables such as velocity and pressure are well predicted with WCSPH, issues may arise in both phases for either negligible dynamics or large dynamics respectively [147].

Deviation in negligible dynamics may arise from the particle disorder of the discrete domain that influences the accuracy of the SPH approximation [127]. Moreover, signal propagation in large dynamics such as impact flows is controlled by the numerical speed of sound which may not be sufficiently large to resolve the forces in a particular inter-particle interaction resulting in either particle clumping or void formation in an area with high kinematic properties.

Both states occur often in multi-phase flows and are a frequently addressed issue. Manenti et al. [138] and recently Ulrich et al. [195] used the XPSH approach of Monaghan [147] with an additional smoothing applied to the particle position at the end of the time step through a smoothed velocity. The rationale of the XSPH approach is to obtain a more regular particle...
position based on the smoothed velocity correction. Additionally, Ulrich et al. [195] used the XSPH formulation to perform pressure smoothing and correct the density for large dynamics flows where the pressure exceeded twice the hydrostatic pressure. However, such smoothing procedures tend to smooth the dynamics of the system such as sharp interfaces and discontinuities.

In this work a particle shifting algorithm has been used originally developed by Xu et al. [212] for ISPH to resolve particle instabilities and clumping due to irregular particle distributions. Xu et al. [212] proposed shifting particles slightly to maintain a regular but still disordered particle distribution by

$$\delta r_i^\alpha = G u_{max} \Delta t R_i^\alpha,$$

where $\delta r$ denotes the shifting distance, $G$ is a constant dependant on the problem with typical values ranging from 0.01 to 1, and $R$ is the shifting vector. Note that the $u_{max} \Delta t$ represents the maximum possible convection distance of the shifted particle. The shifting vector is defined as

$$R_i^\alpha = \sum_j^M \frac{\bar{r}^2}{r_{ij}} n_j^\alpha,$$

with

$$\bar{r} = \frac{1}{M} \sum_j^M r_{ij},$$

where $M$ are the particles within the kernel influence, $n$ is the unit normal vector and $r_{ij}$ the magnitude of the distance between particles $i$ and $j$. Hence, the shifting vector represents the anisotropy of the particle distribution within the support of the kernel, pointing at the direction normal to the unit vector. In addition, the shifted particle hydrodynamic variables are corrected using a second order Taylor expansion. Lately, Lind et al. [117] shifted the particles from high concentration to low concentration areas by using Fick’s law

$$\delta x_i^\alpha = -D \frac{\partial C}{\partial x_i^\alpha},$$

where $D$ is a diffusion coefficient $D = 0.5h^2 / \Delta t$ based on the advection-diffusion von Neumann stability analysis for concentration and $C$ is the particle concentration defined as
\[ C_i = \sum_j^N m_j \frac{W_{ij}}{\rho_j}, \quad (5.15) \]

while the gradient concentration using the symmetric SPH formulation takes the form of

\[ \frac{\partial C_i}{\partial x^a} = \sum_j^N m_j \frac{W_{ij}}{\rho_j} \frac{\partial W_{ij}}{\partial x_i^a}. \quad (5.16) \]

However, particle shifting at a free surface or interface would result in rapid diffusion of the free surface since particles will be shifted from the liquid inner domain to the void kernel support. Lind et al. [117] restricted the shifting of the particles at free surfaces tangent to the free surface by applying the following formulation

\[ \partial_i^{\alpha} = -D \left( \frac{\partial C_i}{\partial s_i^a} s_i^a + \alpha \left( \frac{\partial C_i}{\partial n_i^a} n_i^a - \beta \right) n_i^a \right), \quad (5.17) \]

where \( s \) is the tangent unit vector, \( \alpha \) is the normal direction diffusion parameter and \( \beta \) is a reference concentration gradient at the free surface. The second term is only applicable for negligible dynamics of long duration to minimise unphysical perturbations in the free surface.

The free surface can be located using the divergence of the particle position [109]

\[ \frac{\partial r_i^a}{\partial x^a} = \sum_j^N m_j r_j^a \frac{\partial W_{ij}}{\partial x_i^a}. \quad (5.18) \]

For a particle with a full kernel support the divergence of the particle position equals to 2 for 2-D and 3 for 3-D simulations. A threshold of 1.5 and 2.5 respectively, can therefore be used to define the free surface. Nevertheless, issues can arise in the detection of the free surface specifically with large curvature, flow expansion and splashing.

Skillen et al. [185] modified the diffusion coefficient proposed by Lind et al. [117] based on a von Neumann stability analysis using in addition a constraint based on the velocity magnitude of the particle with a diffusion coefficient \( D_i \) of

\[ D_i = -Ah\|u_i\| \Delta t, \quad (5.19) \]

or

\[ \partial_i^{\alpha} = -D_i \frac{\partial C_i}{\partial x^a}, \quad (5.20) \]

where \( A \) is a problem dependent parameter with values ranging from 1 to 6. Hence for flows involving fast kinematics, the generalised diffusion coefficient of Skillen et al. [185] is
explicitly dependent on the velocity of the shifted particle. This means that each particle $i$ possesses its own local diffusion coefficient.

However, the formulations described herein have only been applied to ISPH formulations. Mokos et al. [144] recently applied the algorithm to WCSPH and extended the surface treatment to 3-D cases using the bi-normal as

$$\dot{r}_i^a = -D \left( \frac{\partial C_i^a}{\partial s_i} s_i^a + \frac{\partial C_i^a}{\partial b_i} b_i^a + a \left( \frac{\partial C_i^a}{\partial n_i} n_i^a - \beta \right) n_i^a \right),$$  \hspace{1cm} (5.21)

where $b$ is the bi-normal to account for shifting in the 3-D surface of free surfaces and interfacial flows. Mokos et al. [144] applied the shifting algorithm to multi-phase gas – liquid flows by applying the interior domain shifting algorithm to the gas fluid and the interior domain and the free surface correction to the liquid fluid. However, the applicability is limited to confined domains only i.e. for interfacial flows where an interface exists throughout the domain.

In this work, the modified shifting algorithm of Skillen et al. [185] (Equation (5.20)) with the surface treatment extension to 3-D of Mokos et al. [144] (Equation (5.21)) is applied to the interior and the free surface or interfacial boundary of the liquid phase since most large dynamics are dominant in the liquid phase [195]. As argued by Mokos et al. [144], correcting the velocity of the shifted particle using a Taylor series expansion has minimal effect of the kinematics [198]. In contrast to the ISPH formulation, the density should also be corrected by applying a Shepard filter. However, applying a Shepard filter at each time step is cost prohibited and leads to smoothing of the density and therefore pressure field.

Similar with XSPH, not applying a velocity and density correction to the shifting deems the method non-conservative since particle velocity and density is not updated at the new position. However, if the Taylor series expansion is applied to the velocity and a Shepard filter to the density; the method will be second order accurate for the velocity and first order for the density.

The failure to correct the velocity and density in WCSPH manifests itself as numerical diffusion that can be described by the minimum particle Peclet number as

$$Pe = \frac{\|u_i\| (u_{\text{max}} \Delta t)}{J},$$  \hspace{1cm} (5.22)
based on the particle velocity $u_i$, the smoothing length $h$ and the Fick’s diffusion. By substituting the diffusion we obtain

$$Pe = \frac{u_{\text{max}}}{AhVC_i}. \quad (5.23)$$

From Equation (5.23) it is shown that as the particle spacing decreases the numerical diffusion decreases. Also, for kinematics with high velocities the numerical diffusion decreases.

Next, the sediment model of the multi-phase model is described. The sediment yield and rheological characteristics are different to the liquid phase, requiring further attention.

### 5.3. Sediment model

The saturated sediment rheological characteristics induced by the liquid flow field exhibit different behavioural regimes that adhere to the sediment properties and shear stress of the liquid phase at the interface. The non-Newtonian nature of sediment flows results from several physical processes such as the Mohr-Coulomb shear stress $\tau_{mc}$, the cohesive yield strength $\tau_c$ which accounts for the cohesive nature of fine sediment, the viscous shear stress $\tau_v$ which accounts for the fluid particle viscosity, the turbulent shear stress of the sediment particle $\tau_t$ and the dispersive stress $\tau_d$ which accounts for the collision of larger fraction granulate. The total shear stress can be expressed as

$$\tau^{\alpha\beta} = \tau_{mc} + \tau_c + \tau_v^{\alpha\beta} + \tau_t^{\alpha\beta} + \tau_d^{\alpha\beta}. \quad (5.24)$$

Accordingly, the sediment phase at low stress state remains un-yielded in that region with the yield strength of the material being greater than the induced stress by the liquid phase and is dominated by the first two terms on right hand side of Equation (5.24). Nevertheless, the saturated sediment stress state should be accounted for. The yield strength of the sediment phase is however unknown and a yield criterion can be employed to evaluate the yield strength of the phase and the sediment failure surface. In this thesis the yield strength of the sediment is modelled using the Drucker-Prager yield criterion and the yielded surface is calculated using the second invariant of the stress tensor.

In a high stress state the sediment is yielded and behaves as a non-Newtonian rate dependant Bingham fluid using the last three terms on right hand side of Equation (5.24). Typically sediment behaves as a shear thinning material with a low and high shear stress state of a
pseudo-Newtonian and plastic viscosity respectively [98]. Simple Bingham models such as the bi-linear or power law Bingham models may account the pseudo-Newtonian region or the shear thinning characteristics respectively only. Herein, the Herschel-Buckley-Papanastasiou model [166] is employed. The aforementioned Bingham model combines the yielded and un-yielded region using an exponential stress growth parameter with a power law Bingham model for the shear thinning or thickening plastic region.

In addition, the generalised Darcy law has been applied in order to correctly simulate the saturated soil motion and the interaction of the sediment and water within the saturated sediment phase. Next, the abovementioned sediment model is described in a physical and mathematical basis.

5.3.1. Yield surface

To determine the state of the sediment SPH particle (yielded or un-yielded region), a yield criterion is used to relate the maximum shear strength of the soil sediment to the hydrodynamic shear strain at the fluid-soil interface. The state of the sediment can be derived directly from the yield criterion dictating its pseudo-fluid behaviour. Above a predefined value related to the shear strength, the sediment is assumed to be at rest whereas below the critical threshold the sediment undergoes yielding.

Considering a simple shear case where no motion in the sediment phase takes place until a critical value of shear stress $\tau_y$ is reached. At the point the fluid stresses acting on the sediment are in equilibrium with the yield strength of the sediment [62] i.e.

$$\sqrt{J_2} - |\tau_y| = 0,$$

where $\tau_y$ is defined as the sum of the Coulomb and cohesive yield strength as

$$|\tau_y| = \tau_{mc} + \tau_c,$$

and $J_2$ is the second invariant of the deviatoric shear stress tensor $\tau^{\alpha\beta}$ defined as

$$J_2 = \frac{1}{2} \tau^{\alpha\beta} \epsilon_{\alpha\beta}.$$

Recalling Equation (5.4), the rate dependant isotropic Newtonian fluid expression for the viscous stresses is written as

$$\tau^{\alpha\beta} = 2\mu \epsilon^{\alpha\beta}.$$
Squaring both sides of Equation (5.4) the following equality is derived

$$\sqrt{J_2} = 2\mu_d \sqrt{II_D},$$

(5.29)

where the term $II_D$ is the second invariant of the strain rate tensor defined as

$$II_D = \frac{1}{2} \varepsilon^{\alpha\beta} \varepsilon^{\alpha\beta}.$$

(5.30)

Thus, the critical threshold for the sediment yielding at the interface can be written as

$$|\tau| \leq 2\mu_d \sqrt{II_D}.$$  

(5.31)

At this point, a yield criterion for the sediment phase is needed to provide the critical value of the sediment shear stress. In this study, the Drucker-Prager yield criterion has been used following previous investigation by Fourtakas et al. [62] on the suitability of different yield criteria.

5.3.1.1. Yield criteria

Total stress models

These yield criteria are expressed in terms of the total stresses and usually include the Tresca and von Mises criterion [170] that apply to un-drained soil behaviour. The Tresca model relates the un-drained strength $S_u$ to the diameter of the Mohr’s circle at failure by

$$f (J_2) = \sqrt{J_2} \cos \theta - S_u = 0,$$

(5.32)

where $\theta$ is the Lode angle associated with the third invariant of the stress tensor $I_3 = 1/3 \tau^{\alpha\beta} \tau^{\theta\gamma} \tau^{\rho\sigma}$. In principal (total) stress space, the function is a regular hexagonal cylinder which has the space diagonal as its line of symmetry as shown in Figure 5.1.

Figure 5.1. Tresca yield surface in principal stress space.
The shape of the function is symmetric in the deviatoric plane due to the permutations of the major and minor principal stresses.

A smoother version of the Tresca criterion is the von Mises criterion which is written using the shear strength of the soil by

\[ f(J_2) = \sqrt{J_2 - a_s} = 0, \]

which is a circular cylinder as shown in Figure 5.2. The shear strength parameter is related to the un-drained soil by the Lode angle either by circumscribing the Tresca criterion at \( \theta = \pi/6 \) or by inscribing the Tresca hexagonal shape at \( \theta = 0 \). This will become apparent shortly in the effective stress models.

![Figure 5.2. von Mises yield surface in principal stress space.](image)

However the aforementioned models describe poorly porous materials such as granular media that depend on the hydrostatic pressure and the effective pressure of the soil skeleton.

**Effective stress models**

To describe general soil behaviour it is necessary to express the yield criterion in terms of effective stress of the soil based on the soil skeleton pressure and the mean hydrostatic pressure. Generally, the Coulomb failure criterion is used to complement the total stress models yielding the Mohr-Coulomb (MC) and Drucker-Prager (DP) yield criterion [30].

The Mohr-Coulomb yield criterion can be expressed in terms of mean pressure and material cohesion strength as

\[ \tau_y = \sigma_m \tan(\phi) + c, \]  

(5.34)
with $\sigma_m$ the mean effective stress, $c$ the material cohesion and $\phi$ the angle of shearing resistance (or angle of repose) [99]. In combination with a trivial Mohr circle exercise the final form of the Mohr-Coulomb yield criterion in terms of the stress invariants can be written as

\[ f(I_1, J_2) = \sqrt{J_2} - \left( p + \frac{c}{\tan(\phi)} \right) g(\theta) = 0, \]  

(5.35)

where

\[ g(\theta) = \frac{\sin(\phi)}{\cos(\theta) - \frac{\sin(\theta) \sin(\phi)}{\sqrt{3}}} . \]  

(5.36)

Similar to the Tresca criterion, the Mohr-Coulomb criterion has a hexagonal form. In principal (effective) stress the function is an irregular hexagonal cone as shown in Figure 5.3.

![Figure 5.3. Mohr-Coulomb yield surface in principal stress space.](image)

Note that for $\phi = 0$ with an un-drained strength $S_u = c$ the Mohr-Coulomb criterion simplifies back to the original Tresca model. However, the corners of the hexagonal planes imply singularities in the yield function and difficulties in the numerical analysis, especially in three dimensions with the use of the Lode angle, i.e. the partial differentials of the yield and plastic potential function [30, 170].

An axisymmetric cylindrical cone yield function can be achieved by replacing $g(\theta)$ by a constant which is not dependent on the lode angle. This model is the so-called Drucker-Prager (DP) yield model that can be represented in principal stress space as a cone shown in Figure 5.4.
By superimposing the MC and DP yield criteria in a deviatoric plane the yield surface of the DP best fit to the hexagonal MC surface is required. Two yield surfaces can be matched at a specific Lode angle $\theta$ of $\pi/6$ and $-\pi/6$ as shown in Figure 5.5. Therefore equation (5.36) can be rewritten as

$$
g(\theta) = \frac{2\sqrt{3} \sin(\phi)}{3 + \sin(\phi)}, \quad (5.37)
$$

at $\theta = \pi/6$ for triaxial extension inscribing the MC hexagon and

$$
g(\theta) = \frac{2\sqrt{3} \sin(\phi)}{3 - \sin(\phi)}, \quad (5.38)
$$

at $\theta = -\pi/6$ for triaxial compression circumscribing the MC hexagon.
In this work the Drucker-Prager model is written in a general form as

\[ f(I_1, J_2) = \sqrt{J_2} + (ap - \kappa) = 0, \]  

(5.39)

where the parameters \( a \) and \( k \) correspond to

\[ a = -\frac{2\sqrt{3}\sin(\phi)}{3 - \sin(\phi)} \quad \kappa = \frac{2\sqrt{3}\cos(\phi)}{3 - \sin(\phi)}, \]  

(5.40)

at \( \theta = -\pi/6 \).

Finally, using Equation (5.31) yielding will occur when the following equation is satisfied

\[ -ap + \kappa \leq 2\mu \sqrt{II_D}. \]  

(5.41)

All models presented herein, assume linear elastic perfectly plastic materials without hardening and softening through the angle \( \phi \) and \( c \). More advance models however can be used such as the Drucker cup critical state model and the Cam clay model [30].

### 5.3.2. Constitutive models

The rheology of the shear mobile layer of the sediment at the interface can be described using viscoplastic rheological laws usually described by Bingham models [98, 176]. The Bingham model is one of the simplest models and provides a satisfactory description of the viscoplastic behaviour of subaqueous sediment flows but given the realities of engineering it cannot approximate all levels of stress, i.e. pre- and post-yield behaviour. Nevertheless, a variety of other Bingham models such as the bi-viscosity and Herschel-Bulkley models are often used in subaqueous flows mimicking the Bingham rheology of a viscoplastic material in low and high stress states [98].

The rheological behaviour of the yielded sediment can be described by a non-linear constitutive formula that relates the shear stress \( \tau^{\alpha\beta} \) of the sediment mixture to the deformation tensor \( D^{\alpha\beta} \) [137], or simply

\[ \tau^{\alpha\beta} = f(D^{\alpha\beta}). \]  

(5.42)

Such a material is restricted to non-Newtonian fluids only with no memory since the shear stress is determined entirely on the deformation tensor at that point. For an isotropic material the symmetrical tensor function of Equation (5.42) takes the form of
\[ f(D^{\alpha \beta}) = \phi_0(I_D, III_D)D^{\alpha \beta} + \phi_1(I_D, III_D)D^{\alpha \beta} + \phi_2(I_D, III_D)D^{\alpha \beta}^2, \]

where \( \phi \) is a scalar function of the deformation invariants. The first term of Equation (5.43) is usually absorbed in the thermodynamic pressure with the total stress tensor defined as

\[ \sigma^{\alpha \beta} = -pD^{\alpha \beta} + \phi_1(I_D, III_D)D^{\alpha \beta} + \phi_2(I_D, III_D)D^{\alpha \beta}^2. \]

However, the above expression is only applicable to isotropic incompressible fluids assuming \( D^{II} = I_D = 0 \) and is applicable to an incompressible mixture flows for small variations of temporal concentration that is usually referred to as the Reiner-Rivlin equation [137]. The Reiner-Rivlin equation is used herein as the template for the non-Newtonian models that are described next.

### 5.3.2.1. Kanatani’s model

Kanatani [100] used the Drucker-Prager model to express the yield stress in terms of the effective pressure and the cohesion of the sediment for incompressible flows using the expression of Equation (5.41) as

\[ \tau_y = ap + \kappa \mu, \]

where \( a = \sin(\phi) \) and \( \kappa = \cos(\phi) \) for two-dimensional sediment flows based on the incompressibility assumption. Using Equation (5.31) the above expression can be re-written as

\[ \phi_1 = \frac{p \sin(\phi) + c \cos(\phi)}{2 \sqrt{I_D}} \leq \mu_{\text{max}}, \]

where \( \phi_1 = \mu_{\text{app}} \) is the apparent viscosity of the sediment phase using a Newtonian approach and \( \mu_{\text{max}} \) is an artificial limiter when \( I_D \rightarrow 0 \) in a pseudo-Newtonian approach (i.e. in impending flow \( I_D = 0 \)). This expression has been used extensively in SPH multi-phase liquid-soil models for the plastic region [57, 138] as the ratio of the soil’s shear strength to the shear rate magnitude resulting in an apparent viscosity as shown in Figure 5.6.

Manenti et al. [138] defined \( \mu_{\text{max}} \) as

\[ \mu_{\text{max}} = 2\eta\mu, \]

with \( \eta = 2 \mu_s / \mu_w \) representing a magnification factor for the sediment dynamic viscosity to mimic the behaviour of a Bingham model. Equation (5.46) is the equivalent of the Levy-Mises perfect plastic equation [137]. However, the singularity at \( I_D \rightarrow 0 \) results in
unphysical behaviour in the sediment phase that manifests itself as creeping. Thus, when $D^{\alpha\beta} = 0$ the stress state leads to an equilibrium but for $D^{\alpha\beta} \neq 0$ with $II_D \to 0$ (i.e. small strain rates) at a flow state may not constrain flow stresses resulting to creeping and therefore an upper limit is needed in terms of maximum viscosity.

Manenti et al. [137] avoid this problem by fixing the position of the un-yielded sediment particles but as observed by Ulrich et al. [195], this may lead to difficulties for the suspended sediment. In addition the low stress state of the sediment is not resolved.

Other models such as the Bingham flow models have been used extensively in sediment transport and subaqueous flows [98]. In this thesis the Herschel-Buckley-Papanastasiou model is used to describe the low and high stress state of the sediment mixture (yielded – un-yielded region), but first a brief introduction to Bingham models is given.

### 5.3.2.2. Bingham models

A Bingham plastic model can be used to model the un-yielded and yielded (shear) layer of the yielded sediment surface as the shear layer exhibits viscoplastic behaviour [176]. In Bingham plastic model the material behaves as a rigid body for stresses smaller than the yield stress and a viscoplastic fluid when the yield stress is exceeded. The first order term of Equation (5.44) for a Bingham fluid takes the following form,

![Figure 5.6. Apparent viscosity using (a) Kanatani’s equation and (b) shear stress plotted against the deformation strain rate.](image-url)
\[ \phi_i = \frac{\tau_y}{\sqrt{\| I \|_D}} + 2\mu_I, \quad \text{for } \tau \geq \tau_y, \]  
\[ \phi_i = 0, \quad \text{for } \tau < \tau_y, \]  
\[ (5.48) \]

and higher order terms are ignored. The basic Bingham model can be extended with the use of a power law exponent to the Herschel-Buckley (H-B) model in the form of

\[ \phi_i = \frac{\tau_y}{\sqrt{\| I \|_D}} + 2\mu I |\| I \|_D^{n-1} |^{\frac{1}{n}}, \quad \text{for } \tau \geq \tau_y, \]  
\[ \phi_i = 0 \quad \text{for } \tau < \tau_y, \]  
\[ (5.49) \]

where the dynamic viscosity is related to the consistency index by

\[ \tau = \kappa \left( \frac{\partial u}{\partial x} \right)^{n-1} \left( \frac{\partial u}{\partial x} \right), \]  
\[ (5.50) \]

or

\[ \tau = \mu \left( \frac{\partial u}{\partial x} \right)^{n-1}, \]  
\[ (5.51) \]

in 1-D space. The exponent defines the behaviour of the fluid for \( n = 1 \) recovering the original Bingham model, \( n < 1 \) pseudo-plastic and \( n > 1 \) dilatant behaviour. The aforementioned model is more detailed than the Bingham model considering the non-linear characteristics of the material. The Bingham and Herschel-Buckley models are plotted in Figure 5.7.

![Figure 5.7. Rheological constitutive relations for a simple Bingham and a Herschel-Buckley model.](image)
Note, that the Herschel-Bulkley, as with the Bingham, exhibits no shear stress before the yield stress point with a singularity occurring when the shear strain is zero. However, the viscous characteristics within the range of elasticity and deformation within the range of low shear rates are significant [130]. Also, fine grained sediment in subaqueous flows exhibit a typical shear thinning behaviour [98]. Therefore, a viscoplastic model with non-linear plastic behaviour with low and high stress state (pseudo-Newtonian and plastic surfaces) must be used to predict correctly the rheology of the shear mobile layer of the sediment at the interface and suspension of the sediment.

5.3.2.3. Herschel-Buckley-Papanastasiou

In this work the Herschel-Bulkley-Papanastasiou (HBP) model [22] has been employed to model the rheological characteristics of the yielded region. The HBP model reads

$$\phi_{\text{HBP}} = \left[\frac{\tau_\gamma}{\sqrt{\pi}} \left(1 - e^{-m\sqrt{\mu}}\right) + 2\mu I_1 \right] \frac{n^{-1}}{\tau},$$

where $m$ controls the exponential growth of stress, $n$ is the power law index and $\mu$ is the apparent dynamic viscosity. Figure 5.8(a) shows the initial rapid growth of stress by varying $m$ whereas Figure 5.8(b) shows the effect of the power law index $n$.

Figure 5.8. Initial rapid growth of stress by varying $m$ and effect of the power law index $n$ for the HBP model.

Note that as $m \to \infty$ the HBP model reduces to the original Herschel-Bulkley model and when $n = 1$ the model reduces to a simple Bingham model.
The HBP model provides information on the pre-yielded and post-yield regions after the apparent yield region defined by the Drucker-Prager criterion with a low stress and high stress region. The sediment phase can also be modelled as a typical shear thinning fine grained material. In addition, there is no need for scale-back methods as used in previous work by other researchers [196].

Accordingly, for a specific skeleton pressure, the inequality of Equation (5.31) defines the yielded surface at that point (or particle). Regardless if the particle is yielded or not the shear stress is calculated using the HBP model with the specific yield stress. However, in the un-yielded region the sediment particles are restricted by setting $Du/Dt = 0$ but discontinuities in the stress summations of the momentum Equation (5.9) do not arise since the viscous stresses in the un-yielded region are computed and assigned to the sediment particle. For the suspended entrained sediment particles a concentration suspension viscosity is used to avoid particle freezing and force imbalance [195].

The advantage of the HBP model is the pseudo-Newtonian region defined by the growth of stress parameter $m$ and the power law index $n$ in the plastic region. This two region approach in combination with the yield criterion has been chosen to model the soil phase without the use of an explicit elastic branch. Nevertheless, successful elastoplastic models have been developed in SPH and applied to sediment transport [26].

### 5.3.3. Sediment skeleton and pore-water pressure

For simplicity, we have assumed a constant critical value of shear stress $\tau_y$ in the HBP constitutive model up to now. This might not always be true for saturated drained conditions. Sediment pressure changes according to the lithostatic conditions and the pore water pressure for a fully saturated sediment. In isotropic, fully saturated sediment under drained conditions the Terzaghi relationship holds

$$P_t = P_{\text{eff}} - P_{\text{pw}},$$  \hfill (5.53)

where subscripts $t$, $\text{eff}$ and $\text{pw}$ denote the total, effective and pore-water pressure, respectively, that can be calculated simply by accounting for the hydrostatic and lithostatic pressures as

$$P_t = h_\gamma_w + h_\gamma_{\text{sat}},$$  \hfill (5.54)
where \( h \) is the height, \( \gamma \) is the unit weight and subscripts \( w \), \( s \) and \( sat \) denote the water, sediment and saturated phase respectively as shown in the schematic of Figure 5.9.

Figure 5.9. Sediment skeleton pressure and saturated sediment pressure schematic.

Unfortunately, Equation (5.54) requires the surface to be tracked in order to determine the maximum height of the saturated sediment which is usually computationally expensive [138]. Instead, the equation of state can be used by modifying the reference pressure dependant on the numerical speed of sound of Equation (4.23) by relating the pore water pressure to the saturated sediment pressure as

\[
p_{pw,j} = B \left( \frac{\rho_{sat,i}}{\rho_{sat,0}} \right)^\gamma - 1
\]

(5.55)

where \( B \) is based on the fluid properties

\[
B = \frac{c_{w,0} \rho_{w,0}}{\gamma_w},
\]

(5.56)

thus recovering the pore water pressure in the saturated sediment even though the density ratio is still based on the saturated sediment.

The total pressure of the sediment is calculated by using Equation (4.20). The sediment skeleton (or effective) pressure can be finally calculated using Equation (5.53). Note that the skeleton pressure can only be applied to fully saturated soils. A partly saturated sediment methodology can be found in [196] or [25].

5.3.4. Seepage forces

In order to simulate the saturated soil motion correctly the interaction of the sediment and water phases within each saturated sediment particle must be taken into account. The
behaviour of saturated soils is determined by the interaction between the soil skeleton and the pore water pressure. When the mixture is deformed the sediment skeleton is compressed and pore-liquid flows through the pores. Water seeping through the pores of a soil produces drag on the sediment phase originating from viscous forces. This force acts on the direction of the water flow [25]. Darcy’s law is often used to describe the viscous drag force as

\[ S = K(u_w - u_s), \]

where \( K \) is based on the soil characteristics and can be written as

\[ K = \frac{n_r \gamma_w}{k}, \]

where \( n_r \) is the porosity and \( k \) is the soil permeability and \( \gamma = \rho g \). The seepage can be added in the momentum equation as an extra term and Equation (4.14) reads

\[ \frac{du^a}{dt} = \frac{1}{\rho} \frac{\partial \sigma^{\alpha \beta}}{\partial x^\beta} + g^a + \frac{S^a}{\rho}. \]

In this work, for simplicity, it is assumed that the water does not flow in the un-yielded region and seepage only acts at the interface of the un-yielded – yielded regions and the interface. Also, the soil mixture is isotropic and fully saturated under drained conditions. In the SPH formalism the seepage force can be added to the SPH momentum equation as

\[ \frac{1}{\rho_i} S^a_i = \frac{1}{2} \sum_j \frac{m_j}{\rho_j \rho_j} K_{ij} \left( \frac{u^a_i x^a_j}{(x^2_j) + 0.01 h^2} \right) x^a_j W_{ij}, \]

where the term \( 0.01 h^2 \) in the denominator is included to avoid singularity as \( x_{ij} \rightarrow 0 \). The seepage force is applied to the yielded region particles \( i \) only for all \( j \) particles irrespective of the phase.

### 5.3.5. Suspension

At the interface, the fluid flow at a sufficient large velocity will suspend the sediment particles in the fluid. This sediment entrainment by the fluid can be controlled through the volume fraction of the mixture by using a concentration volume fraction in the form of

\[ c_{c,ij} = \frac{\sum_{j=2h}^{N} m_j}{\sum_{j=2h}^{N} \rho_j}, \]

where \( N \) is the total number of particles in the mixture, \( m_j \) is the mass of the particle, and \( \rho_j \) is the density of the particle.
where the summation is defined within the support of the kernel. The size of the concentration sampling is chosen as to adhere with the kernel support size of SPH. When a sediment particle is suspended, it is modelled as a pseudo-Newtonian fluid using Equation (5.6). The suspension viscosity can be related to the volumetric concentration by

\[ \mu_{susp} = \mu f(c_v) , \]  \hspace{1cm} (5.62)

where \( \mu \) references to the molecular dynamic viscosity of the fluid and the turbulent contribution from the LES model (Equation (4.40)). This present work uses a suspension viscosity based on the Vand experimental colloidal suspension equation [203] of sediment in a fluid by

\[ \mu_{susp} = \mu_e \frac{2.3c_v}{1.39c_v} c_v \leq 0.3 , \]  \hspace{1cm} (5.63)

assuming an isotropic material with spherically shaped sediment particles. Equation (5.63) is applied only when the volumetric concentration of the saturated sediment particle within the SPH kernel is lower than 0.3 which is the upper validity limit of Equation (5.63). Hence, when a yielded sediment particle volumetric concentration is below the threshold of 0.3 which coincides with the validity of the Vand equation, the sediment particle is treated as a Newtonian fluid, retaining its properties with the exception of the viscosity that follows the Vand Equation (5.63). At that point the particle is entrained by the fluid.

Vand suspension equation also includes the turbulent effects of the LES model through the turbulent viscosity that it is added to the molecular dynamic viscosity similar to the Newtonian phase (Equation (4.40)).

A summary of the model is given at the schematic of Figure 5.10.
5.4. Partial conclusions

In this chapter a novel sediment scouring and transport model was described with emphasis to the rheological characteristics of the sediment phase using the yield surface, the shear layer at the interface and the entrainment of the sediment granulate by the liquid.

The yield surface of the saturated sediment is determined by the Drucker-Prager criterion using the sediment skeleton pressure as the effective pressure of the yield criterion. The yielded region of the saturated sediment is treated as a Bingham viscoplastic fluid by using the Herschel-Bulkley-Papanastasiou constitutive equation. The HBP model has the advantage of describing low and high stress states of the sediment phase with a pseudo-Newtonian and a plastic region respectively, on a shear thinning material. In addition, other closure models such as the seepage force are added to the momentum equation for the yielded saturated sediment phase. The seepage force acts on the sediment granulates as the mixture volume is changing with liquid flowing though the pores inducing a drag force on the sediment grains. Finally the sediment particles eroded by the fluid are modelled as pseudo-Newtonian fluids using a statistical approach with variable suspended viscosity in line with Vand equation.

The current model has been implemented in DualSPHysics [44] SPH solver in the CPU and GPU versions. A description detailed discussion on hardware acceleration, numerical implementation and the DualSPHysics code is given in the next Chapter.
Chapter 6

6. Hardware acceleration using GPUs

6.1. Introduction

This chapter presents the parallelisation and acceleration techniques of SPH using graphic processing units (GPUs). Hardware accelerators have become essential in numerical methods with mainstream industrial and academic CFD codes [6, 164] using high performance computing (HPC) to reduce computational time and cost. The Lagrangian nature of SPH and the absence of implicit time integration render the scheme an ideal candidate for massively parallel computing such as GPUs.

DualSPHysics [44] is an SPH solver consisting of a set of C/C++ codes with CUDA [161] (Compute Unified Device Architecture) extensions capable of running on single CPU (Central Processing Unit) multithreaded cores using OpenMP (Open Multi-Processing) and NVIDIA GPU cards using CUDA. The multi-phase model has been implemented in the C/C++ and CUDA codes to accelerate the numerical computations and reduce computational time.

Herein, the parallel nature of SPH is firstly discussed. In addition, the main hardware architectures that have been used in the past in conjunction with SPH are briefly listed. A detailed analysis of the GPU hardware and CUDA coding techniques provides a clear understanding of the advantages, capabilities and restrictions of the GPU co-processor. Also, the DualSPHysics code structure and architecture is presented with the addition of the multi-phase implementation. Finally, a performance analysis is conducted and compared with a single core CPU processor for the multi-phase implementation.

6.2. Hardware acceleration in SPH

6.2.1. Parallel nature of SPH and n-body simulations

The pair-wise particle interaction of SPH places it within the n-body simulation schemes where traditionally have been used for astrophysical problems [189]. The n-body methods
approximate numerically the evolution of a system of bodies or particles in a discrete domain, where each body interacts in pair-wise manner with every other body in the domain. Examples include Molecular Dynamics (MD) [173], Discrete Element Method (DEM) [211], Vortex Particle Simulations (VPS) [113] and a variety of other particle methods [120].

For an $n$-body simulation with an all-pairs approach (i.e. infinite support in SPH sense) the resulting interactions would be in the order of $O(N^2)$ for $N$ particles. For large computations the all-pairs interaction is unfeasible due to the large number of interaction per particle. Therefore, in a similar manner to SPH, a kernel is used to determine close range interactions only. As mention in Section 4.8.4, in SPH a link-list reduces the numerical particle interactions to $N \log(N)$. Therefore, each particle within the domain interacts with $\log(N)$ particles resulting in a particle-pair algorithm of $N \log(N)$ parallelism since each interaction can be computed interpedently. Consequently, two obvious resulting parallelisation techniques can be applied as shown in Figure 6.1.

![Figure 6.1](image)

Figure 6.1. $N \log T_d$ threads algorithm (a) and $T_d$ threads algorithm with data re-use (b) for parallel $n$-body simulations for $N$ number of particles.

The first technique of Figure 6.1 (a) assigns $T_d$ parallel core threads to each particle equal with the number of interactions of the interpolating particle with its neighbours which equates to $\log T_d$ threads. The overall threads a parallel algorithm may use will be $N \log T_d$ threads. The second technique Figure 6.1 (b) serializes some operations by assigning one thread per interpolation particles in contrast to $\log T_d$ threads of the former algorithm achieving data re-use. Thus, each interpolated particle uses a serial sweep of its neighbouring particles. As a
result, the overall parallel threads used by the system are $T_d$. The advantage of the latter algorithm is not only the data re-use needed to achieve peak performance but also the reduction of the memory bandwidth requirements which constitute the bottleneck in most parallel algorithms [75, 103].

Summarising, $n$-body simulations are well suited to parallel algorithms since the particle pairwise interactions are independent of each other with a finite interpolating support and large number of interacting particles. It is advantageous to use 1 thread per interpolating support and take advantage of the re-use of data and low memory bandwidth requirements to achieve significant speed up.

### 6.2.2. Parallelisation, CPUs and Co-processors

#### 6.2.2.1. HPC with CPUs

Central Processing Units (CPUs) have been in the forefront of HPC for many years [75]. CPU growth and chip size continues to grow according to Moore’s Law [156] that predicts a linear increase of peak performance every 24 months. Currently, the trend of increasing peak performance has moved away from the CPU clock speed due to high power consumption and electrical current linkage and presently is focused to a many threads – many cores architecture which favours parallelism.

When referring to CPUs parallelism, primarily we refer to “instruction parallelism” that it may be taking place on the same core of a CPU under different threads (single chip), on multiple cores of the same CPU (single chip) and multiple CPUs with a single shared memory or within a distributed memory system (multiple chips).

When referring to a shared memory system (a single chip), parallelisation takes place using multi-threaded shared memory over an API (Application Program Interface) such as OpenMP (Open Multi-Processing) comprised of compiler directives, runtime library routines and environment variables. OpenMP uses a fork-join model where the execution runs in a single process over the master thread. The master thread executes in serial until it reaches a parallel region where the master thread splits into parallel threads of the same sheared memory. At this point the thread memory can be shared or be private. Threads are joined back to the master thread when the parallel task has finished and the serial master thread continues consecutively until the next parallel process. A flow diagram is shown in Figure 6.2.
For a distributed memory system (multiple chips), parallelisation takes place using multi-CPU with MPI (Message Passing Interface) between the chips. A private chip memory is located within each CPU and is not shared between the chips. MPI primarily handles the message passing from one CPU to another through a network connection or fast infiniband network communication link. Similar with OpenMP, a master CPU (with a master thread) runs in serial mode until a parallel region is reached. At that point subsections of the process and memory data are copied to individual CPUs. MPI handles the data move from the memory address of one process on a CPU to the memory address of another CPU process parallelising that section of the algorithm as the CPU number increases. In addition, MPI has built-in OpenMP capabilities. Figure 6.3 illustrates the MPI process.

The advantage of the afore mention parallelisation methods and the MPI libraries is clearly the large speed up of the algorithms and semi-implicit directive based parallel algorithm without explicit memory and transfer executions.

Unfortunately, the number of parallel operations does not equate to a linear speed up increase of the overall algorithm.
Amdahl’s argument [4] suggests that the maximum achievable theoretical speed is limited by the serial fraction of the algorithm and can be expressed as

\[
 t_{\text{total}} = t_{\text{serial}} \left[ f_{\text{serial}} + \frac{1}{T_d} (1 - f_{\text{serial}}) \right],
\]

where \( t \) denotes the total and serial time, \( f_{\text{serial}} \) the serial fraction of the algorithm and \( T_d \) the number of parallel threads. In addition, much of the bottleneck in HPC comes from memory bandwidth. The CPU growth may be following Moore’s Law but the memory bandwidth struggles to increase in proportion [68, 103]. The faster the memory the most expensive and limited in size it becomes (i.e. RAM to L1 and registers). Also, the infiniband communications between CPUs causes another bottleneck on the algorithm communication and memory latency.

Large HPC CPU clusters such as a typical IBM Blue Gene/Q [91] are capable of performing 20 petaflops operations per second requiring 7.9 Megawatt of power deeming such large systems not only prohibited expensive due to manufacturing and buying cost but also due to the power consumption and infrastructure large HPC clusters require, such as network communications, air-conditioning and physical footprint. Nevertheless, large CPU HPC has dominated the last few decades. Currently, new emerging technologies promise lower power consumption and cost per flop. These are the so called co-processors and will be described in the next Section.

6.2.2.2. HPC with co-processors

As CPU development and flop per watt is reaching its peak an upcoming technology with massively parallel processors is gaining momentum in scientific computing. Co-processors sit beside a CPU and are used to perform arithmetic, logical operations, etc. accelerating the algorithm performance. These coprocessors manage most of numerical operations reducing the load of CPUs with tasks such as flow control, I/O, etc.

FPGA (Field-programmable gate array) [140] is such a co-processor. FPGAs development started at 1990s with a programmable chip that could handle only arithmetic operations. Nowadays, FPGAs have advanced to independent coprocessors with arithmetic, logic blocks, memory controllers and incorporated serial architecture to handle algorithm flow and serial operations [168]. As the name suggest, FPGAs contain large amounts of logic gates and reconfigurable interconnects that can be programmed and tuned to a specific algorithm.
Therefore, FPGAs have the ability to reconfigure themselves at runtime with very fast memory interconnects and memory blocks. Nevertheless, the hardware advantages come at a programming cost with low level programming languages or HDLs (Hardware Description Language). More user friendly programming techniques such as C language can be used but the explicit configuration of the building blocks deems the method code intensive [140].

Well establish co-processors in scientific computing are the GPU cards. GPUs where originally developed for video output and computing games with programmable capabilities integrated to the GPUs in 2002 [103]. The GPU architecture evolved independently from other multi-processors due to the nature of its function that required fast arithmetic operations for graphics rendering. The latter became achievable with a highly parallel core structure. Modern GPU cards have developed into a scientific tool with dedicated scientific cards of massively parallel nature called either GPU or GPGPU (General-purpose computing on graphics processing units). Each GPU card can hold hundreds of cores with large amounts of memory (2496 cores on a NVIDIA K20X module with 6 GB of memory and 1.31 teraflop double precision instructions with 235 watt of power consumption).

These cores are mainly devoted to floating point arithmetic operations with simplified logic such as branch prediction and in-order execution. Cores are arranged as multiple units with very fast memory bandwidth on chip that are well suited to parallel scientific computing [103]. Programming GPU cards is achievable through either NVIDIA CUDA or OpenCL APIs. CUDA is dedicated to NVIDIA cards only, whereas OpenCL is a framework that can be used in heterogeneous platforms.

OpenCL is supported by many core architectures (ARM, x86, FPGA, GPUs, etc) but can be cumbersome for scientific computing. On the other hand CUDA’s API handles memory transfers from the host (CPU) to the device (GPU) on demand and reduces some of the coding difficulties. Nevertheless, programming GPU cards for scientific computing is a challenge due to the explicit memory management on the device. As with any architecture and co-processors some drawbacks can be identified. The main bottleneck of GPUs is the memory transfer between the host and the device. GPUs were designed for computationally intensive algorithms but underperform for data intensive programs due to their memory architecture and finite memory on the GPU card [103]. However, the massively parallel nature, low power requirements and reasonable pricing make GPU cards a very attractive
alternative to CPUs. A more detailed GPU architecture and CUDA platform discussion is given in Section 6.3.

Recently in 2012, a more traditional co-processor was released by Intel Corporation [92]. The new Xeon Phi co-processor is based on the x86 architecture called Intel MIC (Intel Many Integrated Core Architecture). The advantage of the co-processor is the x86 architecture which is similar to the CPUs and therefore can be utilised through OpenMP/MPI libraries. A typical Xeon Phi co-processor offers 1 teraflop double precision instructions with a power consumption of 225 W over 61 cores with 4 threads each. 4 threads are dedicated to memory control, flow control and I/O management. As with GPUs the main bottleneck is the memory transfer between the host and the device. These co-processors are fairly new and data over performance and optimisation are scattered (for more information and programming techniques see [96]).

6.3. GPU architecture and CUDA programming platform

6.3.1. GPU architecture

A general discussion on co-processors was given in Sections 2.8.2 and 6.2.2.2 where the most common scientific co-processors where briefly introduced. Herein, a more detailed description of GPU cards is provided that includes the streaming multi-processor, memory hierarchy and host – device communication.

6.3.1.1. Multi-streaming processors

The capability of GPU cards to run thousands of lightweight threads in parallel makes them ideally suited to scientific computing. GPUs specialise in compute intensive – high parallel algorithms with limited flow control and data caching in contrast to CPUs where the cache and flow control is a large portion of the chip with a few heavy ALU (Arithmetic logic unit) threads. Figure 6.4 illustrates the two different architectures. At this point it should be noted that the architecture described herein applies to NVIDIA CUDA enabled GPU cards with a compute capability of 3.0 (i.e. NVIDIA Kepler K20).
The basic building block of a GPU is a streaming multiprocessor (or SM) that holds 192 cores with 32 SIMD (Single instruction, multiple data) threads per core. The SMs are designed to execute hundreds of threads in parallel by using an architecture called SIMT (Single instruction, multiple-thread) using extensive thread-level parallelism due to the hardware multithread capabilities. Note, that the total threads per SM are 6144 SIMD threads and modern GPU cards can hold up to 13 SMs per card [161].

The multiprocessor creates, manages and executes threads in groups of 32 parallel threads, called “warps”. One warp is the minimum number of threads number that can be initialised at a time. It should be pointed out that there is no context switching between threads (as with CPUs) and threads run in an out-of-order execution schedule. Each instance of parallel instructions are organised in blocks with 1024 threads per block that occupy the SMs. A block is partitioned in warps containing parallel threads that execute common instructions. The number of threads per block is mainly restricted by the registers thus the “no context switching” between threads. Each SM holds 64 Kb of 32-bit register memory that is equally distributed between the threads with a maximum of 255 registers per thread. Active threads are being executed where inactive threads wait their turn either to receive data and start execution or until another thread to finish so they can take their place [103, 161]. This conquering procedure continues until all threads have finished their SIMT task.

Summarizing, the GPU multi-processor architecture is based on streaming multi-processors that are organised in warps of 32 threads. Each block is divided in warps and is executed on the SMs with a maximum of 32 warps per block. Active blocks are running their parallel task with a single instruction whereas inactive blocks wait either data due to registers restriction or SM usage. This massively parallel nature of GPUs is based on arithmetic operations with limited flow control and memory cache which sometimes may slow down an algorithm due to branching but have superior arithmetic capabilities.
6.3.1.2. GPU memory spaces

Similar to the GPU processors architecture, the GPU memory hierarchy is different to a CPU. There are several memory spaces in a GPU device depending on their intended use. Their characteristics are very distinct with very fast and small memory to large but slow memory with or without cache. These memory spaces are the global, local, shared, texture and registers memory spaces listed according to their speed and size characterises. Figure 6.5 shows the architecture memory spacing with their physical location. The global, texture, constant and local are located “off” the SM chip physically. The global, texture and constant memory are available to any thread of the SM and to all SMs of the same block; whereas the local is private to the thread it is assigned and cannot be accessed by any other thread of any SM. The shared memory and the registers are naturally located “on” the SM chip physically. The shared memory is available to any thread within that SM whereas the registers are private to the thread only.

The memory spaces in a CUDA GPUs have different access latency depending on their location and life span. The closer a memory space is located to the thread the faster the memory space becomes. “Off-chip” to “on-chip” memory has at least one order of magnitude speed difference and the lifetime of the memory decreases as data moves off the chip from a
A detailed list of the memory spaces is given below [161]:

- **The shared memory** it is located on chip with high bandwidth and lower latency in comparison to the local and global memory. Its life time is that of the block and its scope that of all threads in the block with read/write access.

- **The local memory** is located off the chip but its scope is local to the thread. Due to its physical location, memory latency is high and equivalent to the global memory. It is commonly used when register overspill occurs (when there are not enough registers). The threads have read/write access and the lifetime is that of the thread.

- **The texture memory** is located off the chip but it is cached in such a way that a texture fetch costs one device memory read only on a miss or one read from the cache of the texture memory. The memory space was designed specifically for streaming fetches in graphical texture operations. It stays alive for the lifespan of the kernel launch and is available to all threads independently of the block or SM since it is read only.

- **Similar with the texture memory** the **constant memory** is cached off the chip. Fetch cost is similar to the texture memory with the same lifespan, scope and access writes. What makes the constant memory interesting is the way threads can access the constant memory. The memory serialises access to different addresses, therefore when threads access the same address in the constant memory it can be as fast as a register.

- **Registers** are the fastest memory types on a CUDA GPU. This memory space is limited in size and is distributed equally to threads. In general, the registers have minimal latency (most of the time hidden due to the active threads) but it is a large bottleneck due to the limited size and over spilling to the slow local memory. Register lifespan and scope is that of the thread.

This memory architecture is very far from the well established L1/L2/registers architecture of the CPUs. A GPU card should be able to provide data to all threads simultaneously for the size of the block. Memory latency and cycle consumption per operation is the restriction of performance to parallelism [75]. Therefore, the simple structure of a CPU is not sufficient for such a massively parallel architecture. Different memory spaces in GPU cards achieve low memory latency for parallel algorithms. However, as with any architecture there are some advantages and disadvantages that will be discussed in the next Section 6.3.1.3.
6.3.1.3. Advantages, disadvantages and bottlenecks

The multi-threaded parallel multi-processors with fast access memory spaces and a large global memory is undoubtedly a competent platform for parallel algorithms such as $n$-body simulations and SPH. Nevertheless, there are some inherent weaknesses that may be problematic [103]. Data caching and flow control is very limited on the GPU ALU (Figure 6.4) leading to a branching and divergence penalty. Flow control instructions ($if$, $switch$, $while$, etc) force the thread warps to diverge and follow different execution paths that are serialised and increase the instructions executed by the warp and consequently the block. The performance impact of branching is large due to the serial nature of thread diverging. Another restriction of the massively parallel architecture is associated with the register spaces restriction and memory latency. Each SM holds a small finite register size that is distributed to its threads equally. When available register memory per thread is low, the registers tend to free up space by sending data to the local memory attached to each thread. If local memory is low, the register data is over spilling to the global memory. Unfortunately local and global memory is an order of magnitude slower than the “on” chip memory and fetches may be miss-aligned and non sequential after the register overspill. Therefore management of data locality becomes vital in GPU performance. Figure 6.6 illustrates a schematic of the memory bandwidth and memory access cycle speed.

![GPU memory bandwidth and access cycles](image_url)

Figure 6.6. GPU memory bandwidth and access cycles.
A bottleneck of different nature associated with the memory is the communication between the CPU and GPU memories [103]. The GPU physical memory (global memory) is located separately on the card over a PCI Express (PCIe) bus (Peripheral Component Interconnect). CPU RAM memory is located on the motherboard and communication is restricted by the bus speed. However, communications can be hidden while large compute task are running to cover the PCIe transfer latency.

Concluding, the GPU architecture is being designed for large parallel compute intensive task with minimal flow and cache control. Such parallel architecture is specifically suitable to SPH but bottlenecks may occur if the data locality is not managed explicitly. Small blocks per SM tend to reduce the register usage and reduce registers spilling to slow “off” the chip memory. Divergence and branching can have a large impact on the performance since branching is being serialised over a warp with minimum prediction. Simulations such as multi-phase flows suffer from such restrictions [144] and will be discussed in detail in Section 6.5.

### 6.3.2. CUDA programming platform

NVIDIA released a general purpose computing platform to exploit the parallel computing engines of GPUs. CUDA uses C/C++ as a programming language platform for the developers. CUDA employs two programming interfaces, a CUDA runtime and CUDA driver API interface. The API driver is a low level driver interface with the CUDA runtime environment handling implicitly the context, memory and execution management. As a result the CUDA platform simplifies the device management and kernel setup, execution, hierarchy of thread groups and memory spaces.

A kernel is a C function that is being executed on the device [103]. A CUDA kernel should not be confused with the SPH kernel, the CUDA kernel is a C function of the algorithm. Also, note that the CPU is referred as the host and the GPU as the device throughout this thesis. There are three types of kernels, a host, global and device kernel. A host kernel is executed on the host and can be called from the host only. A global kernel is executed on the device and can be called form the host (in CUDA 5.5+ a global is callable form device in addition to the host). Finally a device kernel is executed on the device and is callable from the device only. When heterogeneous programming techniques are used, CUDA assumes that the host and device memory is separate. Therefore the parallel code or kernel executes on the GPU where the CPU handles the serial version of the code with memory copies from the
device to the host and vice-versa for synchronisation of data. Another programming technique copies all the data onto the GPU card and only the program flow (kernel executions) and I/O operations are performed by the CPU. Consequently, the data “live” in the GPU and are downloaded to the CPU when needed for output purposes. Such a programming technique is used in DualSPHysics to avoid latency through the PCIe bus [54]. A more comprehensive description of the DualSPHysics code is given in the next Section 6.4.

6.4. DualSPHysics code

6.4.1. Background

The DualSPHysics code [44] is a set of C/C++ CUDA hybrid codes capable of running on a CPU with OpenMP support and a NVIDIA GPU using CUDA. DualSPHysics was developed from the SPHysics [72] FORTRAN code project which was a collaborative effort by the Johns Hopkins University (U.S.A.), the University of Vigo (Spain), the University of Manchester (U.K.) and the University of Rome La Sapienza (Italy). The aim of DualSPHysics is to minimize computational time and become applicable to real life engineering and environmental problems. Currently, the code is being developed by the University of Vigo (Spain) and the University of Manchester (U.K.).

The DualSPHysics code structure is similar to the parent code SPHysics. Up to this point most of SPHysics capabilities have been implemented to DualSPHysics and others are scheduled to be implemented in later versions. The GPU version of DualSPHysics achieves on average a speed-up of x56 as reported by Domínguez et al. [54] (depending on the hardware) compared with a single core – single thread CPU on the single-phase algorithm. Typically multi-phase codes tend to be cumbersome and slow in performance due to the extra calculations required for the two phases and extra terms in the governing equations. Therefore, a GPU (or parallel) implementation is essential in real life multi-phase applications.

The current multi-phase implementation uses DualSPHysics v3.0.1 with support for compute capability 3.0 (Kepler) GPU architecture. The multi-phase model has been implemented in the C/C++ and CUDA codes for comparison between the two architectures and speed-up purposes. The DualSPHysics code structure and coding techniques are described in the subsequent Sections.
6.4.2. Code structure

A very similar code structure to the SPHysics code has been maintained in DualSPHysics. In the new C/C++ code different programming techniques have been used through the C/C++ libraries to maintain a modular approach such as with SPHysics through templates that are generated during the build and link of the code using C/C++ object oriented capabilities.

The CPU and GPU implementations are highlighted in Figure 6.7, with the initialisation of the code remaining the same in a serial approach for both architectures. The solver portions of the code for the GPU and CPU is also highlighted in Figure 6.7.

![Flow chart diagram for the CPU and GPU code of DualSPHysics.](image)

6.4.2.1. Data handling and I/O

The case initialisation and file output of the code rest on the CPU C/C++ code for both solvers. Case initialisation and read operations are performed in the beginning of the code
from input files originating from the pre-processing tool *GenCase*. Input files are comprised of a XML type file that contains simulation parameters and reference values and a binary file with the particle geometrical information and properties of the particles points. At initialisation the particle position and density for each particle are copied to the main code arrays either for the CPU or GPU version of the code.

6.4.2.2. Neighbouring list

After the case has been initialised and loaded in the algorithm a neighbour list is generated in the first time step and every subsequent time step. The CPU and GPU neighbour lists share the same linked-list scheme described in Section 4.8.4 but there are several differences between the CPU and the GPU versions. The CPU generates a linked-list by initially dividing the domain in square cells of size $2h$ and reorders the index of the particles according to the cells. Finally the particles are assigned to a cell and all arrays with physical properties are reordered in the same manner to match the position index of the new ordered array [54].

In the GPU, a single memory transfer copies the particle data to the GPU. From this point on all operations are performed on the GPU (see Figure 6.7). The linked-list scheme is the same as with the CPU but additional operations are executed to utilise data locality in the GPU architecture. Therefore, a reordering of the particle is taking place where particle memory address is sequential and in line with the surrounding cell particles. The “Trust CUDA” algorithm is used in DualSPHysics to ensure memory address alignment in sequential order [54]. The memory address locality importance has been discussed in Section 6.3. Finally the linked list cell size on a GPU has been reported to be more efficient using $h$-sized cell instead of $2h$ when the particle number is large enough for 3-D simulations. The reasoning behind the cell size is the parallel nature of GPUs where more threads with smaller blocks are used reducing the registers occupancy (see [55]).

6.4.2.3. Particle interaction

After generating the neighbour list the solver moves to the particle interaction for both algorithms. Here the force computations are performed in an SPH sense and therefore each particle interacts with all neighbouring particles within the support domain. On the CPU implementation due to kernel symmetry and kernel derivative asymmetry when a force computation is taking place the pair-wise interaction is performed only once since the force of the first is equal but of opposite sign to the second reducing the computational overhead.
On the GPU the parallel architecture is being explored. Each thread is assigned to an interpolated particle. The interaction of this particle with the support is serialised (i.e. the pair-wise interactions with that particle only) exploiting the advantage of memory address alignment and data re-use as it has already be shown in Figure 3.1 (see Section 6.2.1). Unfortunately the pair-wise interaction symmetry and kernel asymmetry cannot be applied on GPUs since there is no content switching between threads as mention before. Thus, each thread interacts between its support particles only and the pair-wise neighbour particle thread is not allowed access its contents on the register. Due to the large amount o threads that exist on the SM, the overhead of the counter-interaction remains hidden [54].

6.4.2.4. System update
In the system update the time integration and motion of synchronisation of moving boundaries is taking place (see Section 4.8.1). On the CPU algorithm the time integration is performed for every particle and the new time step is computed. On the GPU implementation, the process is parallelised and the new time step is calculated using a CUDA reduction algorithm [161].

6.4.2.5. Pre-processing and Post-processing
DualSPHysics is a complete SPH CFD package with Pre and Post-processing tools. The Pre-processing tool GenCase is a versatile case generation tool for DualSPHysics. It can be used to import geometries form CAD files or geometries can be defined directly through GenCase. It uses an XML type input file where the geometry and parameters of the simulation are defined and outputs the definition XML and a binary file that holds the particle points and properties. These two files are loaded to DualSPHysics for the case initialisation.

In addition to the Pre-processing tools, a number of Post-processing tools exist to assist with visualisation and measuring. These include the Part2VTK tool that exports binary VTK files, IsoSurface tool that creates ISO surfaces for visualisations and Measure tool to probe pressure, density, etc properties at specific points [44].

6.5. Multi-phase model implementation

6.5.1. Issues of Multiphase implementation
A single-phase SPH solver benefits from the GPU implementation with the reduction in the computational time. It deems the code suitable for engineering applications and academic
research purposes. However, real life engineering applications usually involve more than one phase such as air, vapour, liquid and solids. Therefore, multi-phase physics play a crucial role in many applications. In this work a liquid – sediment multi-phase model has been implemented.

The multi-phase implementation to the DualSPHysics GPU code can benefit from the performance of the parallel architecture of GPUs. On the other hand, the resources required for multi-phase implementation are more demanding than a single-phase in a computational and numerical sense. In a computational sense, the phases naturally require a larger number of variables and arrays to be saved on the device increasing the memory requirements on the global memory. Interactions between phases usually require special treatment that may lead to branching and the flow control of the algorithm. In addition, each phase accommodates different physics usually involving branching and extra computations. The parallel nature of GPUs with minimal flow control and cache with streaming multi-processors (SIMT architecture) may pose issues on the overall speed up of the computation. For that reason, a more detail description of the techniques used for the multi-phase implementation is given below. The array structure modifications to accommodate an extra phase, namely the solid sediment phase is described followed by the SPH interaction of particles in the compute forces Section of the solver. Finally, techniques for potential new CUDA kernels are also explained.

6.5.2. Modification of the array structure of SPH

The single-phase DualSPHysics array structure uses an ID array to index the particle order when a case is initialised at the beginning of the simulation. The ID array index remains constant throughout the simulation to assist in the neighbour list. In the multi-phase implementation it is necessary to track the particle phase in addition to the index of each particle. Therefore, a new array IDM is added where the index refers to the particle ID index and the corresponding value of the index to the phase i.e. the liquid phase corresponding value is 0 and 1 for the solid phase. As a result, the array is populated with 0 or 1 values that point directly to the phase of a particle $p$. The IDM array is reordered in the neighbouring list in a similar manner to the ID, position, density, etc., arrays. Since the properties and constants of each phase are different (i.e. reference density of each phase) a list of two index arrays are used that hold the two different phase properties and constants. For example the reference density for each of the two phases may be written as $Rho_{ref} = [1000, 1500]$ with
position index 0 holding in memory the liquid density (i.e. 1000 kg/m$^3$) and position index 1 the saturated sediment density (i.e. 1500 kg/m$^3$). Hence, depending on the particle $p$ index and performing a memory operation the reference density of each phase is recovered as

$$Rho[p] = Rho\_ref\[IDM[p]\].$$

(6.2)

A sample code is given in Figure 6.8 for the calculation of the equation of state. The memory operation performed is cheap computationally since it is located in cached, aligned and sequential-order memory address. Most importantly expensive thread branching is avoided. The only overhead in comparison with the single-phase algorithm is the memory reserve for the new arrays of IDM and phase properties such as the constants. These arrays with the exemption of IDM have small memory footprint holding only two values for a two phase flow. Besides the memory footprint, register usage is minimal in comparison with the branching algorithm of Figure 6.8 (a).

```c
//Phase info
float rrhop=rhop[p];
float rrhop=rhop_soil[p];
float ctebp;
float rhoph0;
float gammap;

//Eos
float pressp;
if (water)
    ctebp=ctebp_water;
    rhoph0=Rho\_ref\_water;
    gammap=gamma\_ref\_water;
    pressp=ctebp*(powf(rrhop/rhoph0,gammap)-1.0f);
else if (soil)
    ctebp=ctebp_soil;
    rhoph0=Rho\_ref\_soil;
    gammap=gamma\_ref\_soil;
    pressp=ctebp*(powf(rrhop/rhoph0,gammap)-1.0f);
else
    pressp=pressp;

//Other operations
float3 v=vel[p];
pressp=make\_float4(r,r,r,r,pressp);
velhop0=make\_float4(r,r,r,r,rrhop);
ace[p]=(p*cb\_gamma, make\_float3(0,0,0): gravity);
```

(a)

```c
//Phase info
float rrhop=rhop[p];
unsigned pp=IDM[p];
float ctebp=ctebppp);
float rhoph0=rhop[p];
float gammap=gammappp);

//Eos
float pressp=ctebp*(powf(rrhop/rhoph0,gammap)-1.0f);

//Other operations
float3 r=vel[p];
pospres[p]=make\_float4(r,r,r,r,pressp);
r=vel[p];
velhop0=make\_float4(r,r,r,r,rrhop);
ace[p]=(p*cb\_gamma, make\_float3(0,0,0): gravity);
```

(b)

Figure 6.8. Sample pseudo-code using (a) a generic approach and (b) using IDM array to avoid branching and reduce register occupancy.

Nevertheless, a multi-phase implementation occupies larger areas of memory on the CPU and GPU since additional arrays are needed for the viscosity, pressure, shear stresses,
concentration of mixture, etc., which are equal in size with the number of particles and in many cases double the memory requirements in comparison to a single-phase code.

The same technique is applied throughout DualSPHysics code when phase selection appears, for instance the particle interaction Section of the code where the forces are computed in an SPH manner. The next section examines the multi-phase implementation of the force computations in the particle interaction section of the code which is arguably the most time consuming element of the algorithm (see Section 6.6).

6.5.3. Modification of the force computations

The force computations Section of the code is of essential importance. Herein, the discretized Navier-Stokes equations and closure models are computed. In a multi-phase model, that requires different constants as shown in Figure 6.8 with dissimilar forms of the governing equations and closure models depending on the phase. In addition, since viscous forces are important at the interface the solver must account for the viscous state of each phase. A different consideration arises at the interface since it is very common to have both phases within a linked-list cell that can lead to severe branching.

When dissimilarity of the governing equations is encountered, a typical CPU (serial) approach is to use if statements to distinguish between phases using the ID array. On a GPU implementation, flow control leads to branching with severe time computation penalty and must be avoided. Therefore, the IDM array can be used on the interpolating and neighbouring particles to reduce the extra terms of the governing equations. Using the new array structure described in Section 6.5.2, depending on the phase of the particle (i.e. fluid or soil) the particle can be either 1 or 0 on the IDM array. Two reference index arrays $S_{\text{Water}}$ and $S_{\text{Soil}}$ (where the $S$ stands for switch) hold opposite unit value arrangements as shown below

$$S_{\text{Water}} = \begin{bmatrix} 1 \\ 0 \end{bmatrix},$$

$$S_{\text{Soil}} = \begin{bmatrix} 0 \\ 1 \end{bmatrix}.$$  \hspace{1cm} (6.3)

Given that a fluid particle in the IDM array holds the value of 0 always, the result is 1 for the $S_{\text{Water}}$ and 0 for $S_{\text{Soil}}$ array. The opposite holds for the soil phase with IDM value of 1. This switch can be used to multiply extra terms of the Navier-Stokes equations to switch on or off their effect depending on the phase of the particle. The index arrays reside in the
constant memory with minimal access time and cheap memory access cycle. Given that GPUs are designed for intensive compute tasks the overhead of this technique is small comparing to branching of threads which leads to serialisation of the algorithm due to poor flow control. A different approach to the current implementation is the creation of different linked-lists for the fluid and soil with a third particle list for the interface. This approach is described by [144] and yielded satisfactory results but it increases the complexity of the code and depends greatly on the size of the interface of the two phases.

![Interaction_forces](image)

Figure 6.9. Schematic of (a) the single and (b) multi-phase interaction forces function.

In multi-phase models such as the current fluid-soil model, the viscous effects at the interface and shear zone of the soil are essential as discussed in Section 2.7. Consequently, resolving such forces must be treated in a parallel manner with minimal impact on the performance. Due to the small number of registers and the over spilling that may occur for large kernels. The kernels functions should be small in size with minimum usage of registers. Figure 6.9 shows a schematic diagram of a single-phase and the current multi-phase implementation. The multi-phase implementation is considerably more expensive in computational overhead because of the extra kernels functions required. To achieve a better performance curve and reduce register occupancy per thread the kernels functions of the multi-phase implementation are reduced in size with the pressure and viscous contribution of the momentum equations calculated in different kernels functions with a non-pair-wise CUDA kernel between then for
the strain rate and turbulence model calculation. The advantage is smaller register usage per thread for each CUDA kernel.

6.5.4. Additional CUDA kernels

Closure models that do not require a linked-list, such as the equation of state and yield criteria, can be handled analogously to Figure 6.8 (b) since it is a parallel execution and does not require pair-wise interaction through a linked-list algorithm. When a linked-list is required for the pair-wise interactions the computational cost increases for the reasons described above. In general, such CUDA kernels require a small number of registers and do not cause bottlenecks in the algorithms.

Concluding, the multi-phase implementation in DualSPHysics uses a different array structure by employing a master IDM array and smaller reference two-index arrays to perform fast memory reference operations instead of flow control instructions such as if statements. The gain in performance comes from the minimal branching achieved. In addition, the reduction in size of the CUDA kernels improves the register usage of the threads. Next, a performance analysis is conducted in comparison with a serial CPU architecture and an overview of the performance of the GPU code is carried out.

6.6. Performance analysis

6.6.1. Serial - parallel run time comparison

A parallel to serial speed up test has been conducted to determine the speed up characteristics of the GPU parallel code. The speed up curve also provides information on the scalability of the multi-phase implementation. The GPU test was performed on an NVIDIA Kepler K20 GPU card with 5 GB memory where as the CPU runtimes where recorded on an Intel i7 2.8 GHz processor with 4GB of memory.

Figure 6.10 shows the achieved speed up curve for the parallel to serial comparison over an average of three runs. The maximum speed up achieved over the serial code (single CPU using a single thread) is 58 which is a satisfactory result considering the non-parallel sections of the code such as the double summation of the stress formulation compared with work by other researchers such as Mokos et al. [144].
However, the scalability of the GPU implementation is limited to around 1.6 million particles. The low scalability of the GPU implementation gives rise to the need for multi-GPU implementations. Domínguez et al. [53] made use of several GPU cards over MPI communications with excellent scalability results for up to a billion particles. Such multi-GPU implementation could be potentially a method in increase the number of particles to the tens of millions.

It should be noted here that the speed up reported are conducted with a serial and single threaded algorithm. A fair GPU-CPU comparison would require the use of the OpenMP library for the CPU algorithm. Nevertheless, the since the architectures are dramatically different the speed up over the serial algorithm reported herein are a good measure of the parallel capabilities of the GPUs.

**6.6.2. GPU computational time map**

Figure 6.11 and Figure 6.12 show the percentage of the total computational time of the GPU algorithm that is divided depending on the function they performed, grouped in the compute forces (FC), system update (SU) and neighbour list (NL) portions of the GPU algorithm.

It is directly evident that the compute forces group is the most demanding portion of the code since all particle interactions are computed within the FC part of the code. This part of the
code includes the EOS calculations, the density and momentum equation approximation including the viscous forces and the yield criteria calculations. Since these arithmetic operations require a large amount of registers and memory, bottlenecks may occur. Indeed, the system update that can be parallelised readily, since only a handful of flop are required with low register utilization, requires less than 1% of the computational time.

By increasing the number of particles, the computational map of Figure 6.12 shifts towards the neighbour list by almost doubling the computational resources when the scalability reaches a minimum. The SU is still quite small occupying only 1% of the total computational time.
Figure 6.11. Percentage of the runtime taken by each part of the GPU code for 26,000 particles. The symbols denote: CF = Compute Forces, SU = System Update, NL = Neighbour List.

Figure 6.12. Percentage of the runtime taken by each part of the GPU code for 1,600,000 particles. The symbols denote: CF = Compute Forces, SU = System Update, NL = Neighbour List.
6.7. Partial conclusions

In this Chapter the parallel $n$-body nature of SPH has been discussed. The parallel implementations of SPH include CPU and other co-processors implementations including FPGAs, the Xeon Phi co-processor and GPU implementations. Since the power consumption of the CPU based architectures is increasing constantly, low power consumption co-processors are becoming increasingly popular. GPUs are ideal for $n$-body simulations due to the massively parallel architecture with relative low purchase cost and power consumption per flop. However, the GPU architecture has limited flow control with explicit memory management. Branching and (memory) register occupancy has been at the forefront of GPU development as a drawback. In this thesis, the multi-phase model was implemented in the CPU and GPU branch of DualSPHysics and a direct comparison yielded a speed up of 58 in comparison with the single-thread serial code. This was achieved by avoiding branching using memory operations that are computational cheap on GPU cards and are mostly hidden by the off-the-chip memory latency. It was noted that the major bottleneck of the code is the “force computation” function. Remedies to speed up the algorithm further is the reduction of the size of the CUDA kernels and the creation of separate linked lists for each phase as reported by other researchers such as Mokos et al. [144].
Chapter 7

7. Validation cases and applications

7.1. Introduction

This Chapter presents the validation and verification of the two-phase liquid-sediment model. The liquid and sediment model is validated independently with static and dynamic cases and is compared to numerical and experimental results available in the literature.

Initially, the capabilities of the liquid phase are demonstrated followed by single and two phase sediment flows. Where possible, each phase of the multi-phase model is examined independently from the other, such as the yield criteria selection and constitutive equations. A variety of 2-D cases are presented finishing with a 3-D case for verification of the model. First, the 2-D liquid phase validation is presented.

7.2. 2-D validation cases

7.2.1. Liquid phase

In this Section the liquid phase predictive accuracy is investigated for a single phase fluid flow. Using the WCSPH SPH code DualSPHysics [44] the applicability and improvements of the δ-SPH, particle shifting algorithm and viscous formulations is demonstrated. However, since DualSPHysics is already a well validated SPH code [44] only the specific choices (δ-SPH) and new implementations (viscous formulation and particle shifting) are validated for the liquid phase. In addition, the rationale for the specific model choices is proven.

7.2.1.1. Droplet impact on a flat plate

δ-SPH

To demonstrate the effectiveness of the δ-SPH algorithm as a diffusion term included in the continuity equation (see Section 5.2.2) under violent impact flows and the spurious pressure field due to the stiff equation of state in WCSPH (Equation (4.20)) a test case with a droplet impacting a horizontal plate is employed [124].
The radius of the 2-D sphere is 0.00085 m impacting on a flat plate of 0.0085 m length with a particle spacing of 0.00002 m resulting in a total of 9835 particles. The liquid droplet has a reference density \( \rho_l = 1680 \text{ kg/m}^3 \) with a viscosity of \( \mu_l = 6.4 \times 10^{-3} \text{ Pa s} \) without gravity. Two different configurations have been used for the density filtering. The first uses a Shepard filter with a filtering frequency of 50 time steps whereas the second uses \( \delta \)-SPH with a density diffusion coefficient of 0.1 [145].

<table>
<thead>
<tr>
<th>Time (( \mu \text{s} ))</th>
<th>Shepard filter</th>
<th>( \delta )-SPH</th>
<th>Experimental [124]</th>
</tr>
</thead>
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<td><img src="#" alt="Experimental" /></td>
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<tr>
<td>180</td>
<td><img src="#" alt="Shepard filter" /></td>
<td><img src="#" alt="( \delta )-SPH" /></td>
<td><img src="#" alt="Experimental" /></td>
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<tr>
<td>240</td>
<td><img src="#" alt="Shepard filter" /></td>
<td><img src="#" alt="( \delta )-SPH" /></td>
<td><img src="#" alt="Experimental" /></td>
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Figure 7.1. Comparison snapshots of the pressure field of a droplet impacting a flat surface using a zeroth-order Shepard filter and \( \delta \)-SPH diffusion term with experimental results droplet profile [124].
Since the numerical model is single phase and with the absence of the gas phase and to avoid deformation of the droplet, the wall has been given a constant velocity of 2 m/s in the vertical direction. Figure 7.1 shows snapshots of the simulations at various time steps.

This single phase simulation does not include surface tension. However, the spreading characteristics of the droplet are not greatly influenced by surface tension but rather the inertia of the droplet at large Reynolds numbers [16]. In this test case the Reynolds number was 8700 based on the radius of the sphere with a Weber number of 970.

In Figure 7.1(b-d) the droplet has impacted the plate with the impact pressure wave propagating through the droplet. However, in WCSPH the stiff equation of state results in spurious pressure field due to the density change (usually 1%). Comparing the zeroth order correction with the diffusion mechanism of δ-SPH it is shown that despite the zeroth order filtering of the density a spurious pressure field is still present. The diffusion term of δ-SPH on the other hand has suppressed the spurious pressures in comparison to the Shepard filter. This is clearly demonstrated in Figure 7.1(d).

It should be mentioned at this point that the diffusion term of δ-SPH in Equation (4.30) is based on an artificial diffusion parameter that uses the density difference and the normalised distance of two particles. Therefore, care should be taken when tuning the δ-SPH constant parameter δ_d to avoid over-diffusion of the density.

**Particle shifting**

Particle shifting algorithms have generally been employed in ISPH [117, 185] to stabilise the computation by smoothing the particle distribution from high to low concentration areas of the domain. Lately, particle shifting was employed for WCSPH multi-phase gas-liquid flows [144] in an attempt to correct the troublesome non-expanding gas phase in air entrainment phenomena. In this thesis, the particle shifting of Lind et al. [117] and Skillen et al. [185] has been used in a similar manner to the ISPH formulation for the liquid phase. At high velocity impact flows and wave fronts especially between different phases with large density and viscosity ratios, spurious pressure oscillations can be observed. Particle clumping at these fronts is common altering the pressure field and thus the dynamics of the flow front with unphysical voids. To evaluate the effectiveness of particle shifting algorithm the droplet test case using δ-SPH is compared with a simulation that uses δ-SPH and the particle shifting of Section 5.2.3. Figure 7.2 shows a comparison of the two configurations and the effects of the particle shifting algorithm on the formation of unphysical voids.
Figure 7.2. Effect of particle shifting algorithm (a) on the particle distribution and pressure field of the domain at $t = 370 \, \mu s$ in comparison to (b) only $\delta$-SPH.

Figure 7.3. The (a) pressure and (b) velocity profile of the droplet at initial contact with the plate and comparison between $\delta$-SPH, $\delta$-SPH + shifting algorithm and VoF numerical results [124].
The void structure of Figure 7.2 (b) is created due to particles following the streamlines after a sudden impact of the droplet to the plate creating particle line structures (stacks of particle lines following the streamline) that eventually collapse after particle clumping has occurred. This unphysical void and particle clumping phenomena disturbs the pressure field either by a spurious pressure field or by pressure waves originating around the void. This can be clearly observed in the comparison of Shepard and δ-SPH density correction of Figure 7.1(c) where an oval shape high pressure area near the top of the droplet is created that is finally manifested as voids at later time in Figure 7.2 (b). In addition to the void formation effect of shifting, the pressure field of Figure 7.2 (a) is considerably smoother and symmetric. Since the surface shifting of Lind et al. [117] is employed the surface of Figure 7.2 (a) is smooth maintaining an angular shape.

However, since particles are shifted from their original position based on Fick’s law, at impact – keeping in mind that in WCSPH small compressions of the order of 1-2% are permitted through the equation of state and the numerical speed of sound – the compressed particles should in theory shift from the high concentration impact zone to the lower concentration interior droplet domain.

This redistribution of particles from the high to the low pressure zone will alter the pressure field in the droplet transmitting the pressure wave and therefore reducing the compressibility of the fluid. Evidence of this assumption is given in Figure 7.3 where a comparison of the pressure field with and without δ-SPH and a Volume-of-Fluid (VoF) simulation is shown [124].

However, the shifting algorithm may introduce some numerical diffusion to the simulation since particles are shifted numerically to different positions. A measure of the minimum numerical diffusion is given in Equation (5.22) using the minimum particle Peclet number. For a convection velocity of \( u_{\text{max}} = 2 \) m/s and assuming the maximum concentration gradient change is located at the free-surface curvature of the droplet the minimum particle Peclet number is \( \text{Pe}_{\text{min}} = 40 \) which can be interpreted as a convection dominated flow.

Next, a well known SPH validation case is used to access the viscous forces formulation, δ-SPH and shifting algorithm in the liquid phase using a dam break.
7.2.1.2. Dam break validation

To demonstrate the validity of the liquid phase, the dam break test case has been chosen. The dam break case is a typical SPH validation case for gravity – pressure driven flows with non-linear free surfaces.

In line with available experimental data [105], a container with a length of $4L$ and height of $3L$ contains a column of water with height $H = 2L$ and length $L = 1$ m. The water column is located in the left hand side supported by a gate which is suddenly removed to start the water column collapse. The liquid impinges on the right hand side wall with an impact pressure increase and fragmentation of the free surface. A schematic of the dam break test case in given in Figure 7.4. An initial particle spacing of $dx = 0.01$ m produced 22 000 particles.

![Schematic of the dam break test case with $L = 1$ m.](image)

The liquid is considered as water with a density of $\rho = 1000$ kg/m$^3$ and a dynamic viscosity of $\mu = 10^{-3}$ Pa s. The $\delta$-SPH diffusion coefficient was 0.1 which is typical for such simulations [145] with a shifting parameter of $A = 2$. The numerical results are compared with the experimental results of Koshizuka and Oka [105]. Figure 7.5 and Figure 7.6 show the water toe advance and height reduction as the dam breaks to the right hand side in non-dimensional form by

$$
\begin{align*}
t^* &= t \sqrt{\frac{2g}{L}}, \\
x^* &= \frac{x}{L}, \\
h^* &= \frac{h}{2L},
\end{align*}
(7.1)
$$

where $t$, $x$ and $h$ is the time toe position and water height respectively.
Figure 7.5. Dam break toe front comparison between the experimental and numerical results for a particle spacing $dx = 0.01$ m.

Figure 7.6. Dam break height (height decrease of the water column as the dam breaks) comparison between the experimental and numerical results for a particle spacing $dx = 0.01$ m.
Both figures show close agreement with the experimental results with small variations of the water height reduction after $t^* = 4$, which is known to be associated with the dynamic boundary conditions and particle sticking effects that are associated with the current boundary condition. Nevertheless the agreement is satisfactory. Since the dam is breaking progressively due to gravity and the impact pressure field is similar in both cases and is not included in this work.

Next, the saturated sediment phase is examined for a number of test cases in terms of the yield criteria, the pressures of the two-phase model (i.e. mixture and skeleton pressure) and the validity of the constitutive equations.

**Partial summary**

In Section 7.2.1 the predictive accuracy of the fluid was examined for gravity and pressure driven flows. The droplet impacting a flat plate case has been employed to demonstrate the effectiveness of the $\delta$-SPH algorithm through the dissipation term included in the continuity equation under violent impact flows in comparison to the Shepard filter. However, care must be taken when choosing the diffusive coefficient of $\delta$-SPH that may result in over dissipation of the density field. Another essential implementation to the liquid phase was the particle shifting algorithm.

Particle shifting is used in this thesis to stabilise the computation by smoothing the particle distribution from high to low concentration areas of the domain and reduce the void structures created due to particles following the streamlines after a sudden impact of the droplet to the plate creating particle line structures. It has been demonstrated that the unphysical voids are dissipated with a smoother pressure field.

To validate the shifting and $\delta$-SPH algorithm implementation, a dam break test case has been used. The toe advance and height reduction of the dam as it breaks were in good agreement with experimental data. Small deviations from the experimental data in the height of the water column were due to the boundary conditions and the reader is referred to Chapter 8.

**7.2.2. Sediment phase**

**7.2.2.1. Yield criteria validation using Kanatani’s approach**

Kanatani [100] reduced the Drucker-Prager model to an associated flow rule for incompressible sediment flows to the expression of Equation (5.45)
Herein, a similar approach has been used as described in Section 5.3.1 by using the Newtonian constitutive equation written in the following form

\[ \tau_y = \alpha p + \kappa \epsilon. \tag{7.2} \]

or

\[ |\tau_y| \leq 2\mu_d \sqrt{I_D}, \tag{7.3} \]

\[ \mu_{app} = \frac{|\tau_y|}{2\sqrt{I_D}} \leq \mu_{max}, \tag{7.4} \]

where \( \mu_{app} \) is the apparent viscosity of the sediment under yielded conditions and \( \mu_{max} \) is the sediment viscosity before yielding \( \mu_s \). This simplistic methodology of describing the sediment flow behaviour has proven effective by other researchers in SPH [57, 138, 195] and is being investigated herein in terms of the suitability of two effective stress yield criteria models. A comparison between the Mohr-Coulomb (MC) and Drucker-Prager (DP) yield models is given below for static and dynamic cases with qualitative and quantitative comparisons with analytical solutions and experimental results.

However, more complex constitutive equations have been used in this thesis, already discussed in Section 5.3.2.3, and the purpose of this Section is to show the applicability of the yield criteria and yield surface prediction.

**Still sediment-liquid case**

A variant of the still water test case which is a typical SPH validation case has been used to assess the interface of the two phases, the variable viscosity and density in the sediment and liquid. The sediment is located at the bottom of a tank with the liquid phase above at the top in a 2-D case where the length and height of the tank is \( L = 4.0 \) m and the height of the solid sediment is \( H_s = 1.0 \) m with equal liquid height \( H_l = 1.0 \) m as shown in Figure 7.7.

Three different particle spacing configurations have been used to demonstrate the convergence of the case. The particle spacing is varied from \( dx = 0.04 \) m to \( dx = 0.01 \) m resulting in 1446, 5391 and 9500 particles respectively.

The liquid reference density was set to \( \rho_l = 1000 \) kg/m\(^3\) with the saturated sediment reference density equal to \( \rho_s = 1750 \) kg/m\(^3\) following the relationship \( \rho_s = 1.75 \rho_l \). The dynamic viscosity of liquid was set to \( \mu_l = 10^{-3} \) Pa s and the sediment maximum viscosity was set to \( \mu_s = 5 \times 10^3 \) Pa s with the Coulomb parameters \( c = 1000 \) Pa and \( \phi = 45^\circ \). The still sediment-liquid
case was initialized to the reference density conditions of $\rho_s = 1.75\rho_l$ for the liquid and saturated sediment density.

![Figure 7.7. Definition sketch of the domain for the still sediment liquid case.](image)

A convergence analysis based on the Global Relative Error (GRE) which can be used as a measure of the stillness of the sediment after yielding [163] has been performed according to

$$GRE = \sqrt{\sum_{j}^{N} \left( \frac{x_j^{n+1} - x_j^{n}}{x_j^{n}} \right)^2},$$

(7.5)

where $x$ is the position of the particle and $n$ denotes the time step.

The motivation at this point is to assess the stability of the interface between the sediment and the liquid when, in theory, there should be no movement. However, in SPH small tiny movements of particles always occur, hence, this case tests the stability of the yield models embedded within SPH at the interface. Figure 7.8 shows the convergence of the still case using both yield criteria, at different particle spacing. In addition, a steady state threshold ($GRE < 10^{-5}$) has been introduced below which the particles of both phases are said to remain still.

It is shown that below a particle spacing $dx = 0.02$ m initially the GRE increases above the threshold as would be expected caused by the relative movement of the liquid particles due to the SPH error. After $t = 0.6$ s a steady state has been reached. The rearrangement of liquid particles is reduced over time and the simulation returns to steady state. For a particle spacing of $dx = 0.04$ m the GRE remains above the threshold value for both models.
Figure 7.8. Comparison of the Mohr-Coulomb (MC) and Drucker-Prager (DP) yield criteria for different particle spacing using the GRE over time.

The GRE for the MC and DP models appears similar for the fine particle spacing with the exception of some peak values at around $t = 0.4$ s where the MC exhibits small oscillations. Therefore, a ratio of $dx / h = 0.769$ is necessary for a steady state interface based on a kernel smoothing length of $a_s = 1.3$ and particle spacing $dx = 0.02$.

Figure 7.9 shows the viscosity of the still liquid-sediment phase for both models at $t = 0.2$ s with a particle spacing of $dx = 0.01$ m. Notable is the difference of the yield surface thickness of the two models across the mid-depth of the tank in Figure 7.9. Note that at this instant ($t = 0.2$ s) both models exhibit almost the same degree of stillness and hence value of GRE, but the actual yielded surface is different. The DP criterion tends to have a larger yielding interface creating a thicker shear layer between the liquid and the sediment in comparison to the shear layer resulting from the MC criterion. This result appears to be in agreement with the other test cases examined later.

The advantage of this test case is that it illustrates the stability at the interface. The interface remains steady without particle exchange, diffusion or unphysical repulsion. The variable viscosity remains close to the threshold with low or little variations when steady state is reached with the exception of the interaction with the boundaries where small variations occur when the GRE is above the threshold level which is expected.
Figure 7.9. Viscosity of the still liquid-sediment phase for (a) Mohr-Coulomb (MC) and (b) Drucker-Prager (DP) yield criterion at $t = 0.2 \text{ s}$.

**Tangential annular flow between two coaxial rotating cylinders**

The tangential annular flow test case has been applied to validate the rheological parameters of the sediment-liquid mixture. In this shear driven test case, two co-axial cylinders with radius $R_1 = 0.5 \text{ m}$ and $R_2 = 0.1 \text{ m}$ are co-rotating at a steady frequency of $f = 5 \text{ Hz}$ as shown in Figure 7.10. To avoid initial sudden disturbances at the start of the simulation the velocity of the cylinders walls is linearly accelerated to their terminal viscosity over a period of $t = 0.1 \text{ s}$.

The sediment mixture density is set to $\rho_s = 1540 \text{ kg/m}^3$ with a viscosity of $1000 \text{ Pa s}$ and the Coulomb parameters $c = 100 \text{ Pa}$ and $\phi = 45^\circ$. The total number of sediment particles is 3703 with a particle spacing of $dx = 0.02 \text{ m}$.

In 2-D, assuming that the flow is laminar and incompressible at steady state, only the tangential velocity is non zero with no pressure gradient in the circumferential direction, the velocity profile can be found as [18]
\[ u_\theta = \frac{C_1 r}{2} + \frac{C_2}{r} \quad (7.6) \]

where \( C_1 \) and \( C_2 \) are the integration constants.

An \( L_2 \) error for the velocity \( u \) can now be defined as

\[ L_2(u_\theta) = \sqrt{\frac{1}{N} \sum_{j=1}^{N} \left( \frac{u_{\theta,\text{analytical}}^{SPH} - u_{\theta}^{\text{SPH}}}{u_{\theta,\text{max}}^{\text{SPH}}} \right)^2} \quad (7.7) \]

Figure 7.11 shows the \( L_2 \) error for both yield criteria against time. The \( L_2 \) error levels for both cases show a steady convergence within acceptable levels. Apparent is the slow decay in \( L_2 \) error of the DP criterion, in agreement with the still sediment liquid case, whereas the M-C criterion reached steady state faster.

The physical meaning of the slow \( L_2 \) error decay is that the sediment flow is much less viscous in comparison with the MC criterion. Therefore the shear flow induced by the axial boundaries reduced the variable sediment viscosity further than the MC criterion, taking more time to reach steady state. Both yield criteria finally reach the same \( L_2 \) error at steady state at the end of the simulation. Figure 7.12 shows a graphical representation of the velocity distribution after 1 revolution (0.2 sec) and at 10 revolutions (2.0 sec). Note that the DP criterion, evident also by the \( L_2 \) error, at 1 revolution has not reached steady state.

Figure 7.11. Temporal growth of \( L_2 \) error for the Mohr-Coulomb and Drucker-Prager yield criteria for a tangential annular flow.
Figure 7.12. Velocity distribution of the sediment after 1 revolution (a & b) and at the end of the simulation (c & d) after 10 revolutions.

Two-phase tangential annular flow
The tangential annular flow test case has been extended to a two-phase case to investigate the sediment and liquid rheology and interface. The geometrical configuration of the case is identical to the single phase case. However, the outer half of the interior domain of the annulus is filled with sediment particles and the inner with liquid particles.

The liquid and sediment particle spacing and properties are as follows. The particle spacing is set to $dx = 0.0039$ m that resulted in 50,886 particles. The liquid reference density is set to $\rho_l = 1000$ kg/m$^3$ with the sediment reference density equal to $\rho_s = 1.54 \rho_l$. The viscosity of the liquid is set to $\mu_l = 10^{-3}$ Pa s. Two different values of sediment viscosity have been used with $\mu_s = 5 \times 10^3$ Pa s and $\mu_s = 1$ Pa s which is considerably lower. The reason for the second lower viscosity is to test the stability of the behaviour of the interface and investigate the effect on the flow reaching steady state. Since the maximum apparent viscosity is three orders of magnitude lower that the single phase tangential annular flow, both yield criteria should behave in a similar manner. At steady state a uniform axial particle distribution should be observed.

Figure 7.13 shows the $L_2$ error for both yield criteria against time. The $L_2$ error provides a measure of steady state convergence over time. Both yield criteria finally reach the same $L_2$ error at steady state at $t = 1.2$ s. As expected the difference between the two models is minimal for the low viscosity and behaves similar to the single phase for the higher viscosity. In addition a qualitative comparison of the flow field is given in Figure 7.14.
Figure 7.13. The growth of L2 error for the MC and DP yield criteria for a tangential annular flow for 
\( \mu_s = 5000 \text{ Pa s} \) and \( \mu_s = 1.0 \text{ Pa s} \).
The last two cases demonstrate not only the convergence of the $L_2$ error within acceptable levels and the difference between the Mohr-Coulomb and Drucker-Prager models but in addition, the importance of the sediment viscosity value $\mu_s$. Using Kanatani’s approach to acquire a $\mu_{app}$ with a maximum threshold viscosity $\mu_s$ may result in an underestimation of scouring for high values of $\mu_s$ that have been traditional used to avoid creeping [57, 138, 195] but low values will result in overestimation of the scouring phenomenon as proven herein.

Nevertheless both yield criteria produce satisfactory results. It is evident from the still sediment-liquid, the tangential annular flow and the current test case that the MC criterion tends to reduce the shear layer by means of higher variable viscosities when the sediment
particles are suspended trending to a more conservative model which underestimates the erosion profile. The MC criterion may underestimate the yield strength of sediment [99, 218] which is in line with this discussion, whereas, the DP criterion is said to be a smoother alternative to the MC criterion where the yield strength, at least on these specific test cases, is better depicted.

Next, a dynamic scouring test case is examined by using an erodible dam break to evaluate the scouring profile and shear layer with available experimental results.

**2-D Erodible dam break**
The shear layer and suspension of the sediment is qualitatively validated using an experimental erodible dam break [65]. When the liquid column is released, the hydrodynamic stress at the interface induces scouring at the sediment interface. The shear layer induced propagates to some depth where the viscosity, density and pressure changes from their initialized value.

In the computational setup a fully saturated sediment bed with height $H_s = 0.6$ m is located below a dam of liquid with height $H_l = 0.1$ m and length $L_l = 2.0$ m. A definition sketch is shown in Figure 7.15. The particle spacing is set to $dx = 0.002$ m producing 328 353 particles in the domain. The density ratio between the two phases is $\rho_s = 1.54 \rho_l$ with the liquid density $\rho_l = 1000$ kg/m$^3$. The sediment and liquid viscosity was set to $\mu_s = 1000$ Pa s and $\mu_l = 10^{-3}$ Pa s respectively. The sediment Coulomb parameters are set to $c = 100$ Pa and $\phi = 45^\circ$ (in line with Ulrich et al. [196]).

![Figure 7.15. Definition sketch for the 2-D erodible dam break configuration.](image)

Figure 7.16 to Figure 7.19 shows a comparison of the shear layer velocities within the suspension layer of the sediment and compares the experimental results from the Louvain experiment [65] with the MC and DP yield criteria for different points in time of the simulation (at $t$ of 0.25 s, 0.50 s and 1.00 s).
Comparing the two yield criteria the DP yield model shows better agreement in the sediment shear layer with the experimental results. The MC criterion appears to under predict the erosion and the depth of the shear layer. The erosion of the shear layer with the DP model is more distinct with larger depths. In addition, the dunes created by the liquid in the shear layer agree closely with the shape from the experimental results. Also, the liquid peak at the free surface is in closer agreement with the DP criterion. For example the liquid surface peak created by the scoured sediment is dominant in the experimental results and DP criterion for $t = 1.0 \text{ s}$ but absent at the MC model (Figure 7.19).

![Shear layer formation and shear layer velocity field for the Mohr-Coulomb (a) and the Drucker-Prager yield criterion at $t = 0.25 \text{ s}$ and qualitative comparison with the experimental results, not in the same horizontal scale [65].](image)
Figure 7.16 shows a snapshot of the experimental and numerical results at $t = 0.25$ s. It can be clearly observed that the shear layer produced by the DP criterion tends to be thicker with the sediment being transported along after the dam breaks onto the sediment. The shape of the wave front is more closely related to the DP model in comparison with the experimental results. Nevertheless, the MC model shows reasonable agreement with the experimental results.

![Figure 7.17](image)

**Figure 7.17.** Shear layer formation and shear layer velocity field for the Mohr-Coulomb (a) and the Drucker-Prager yield criterion at $t = 0.50$ s and qualitative comparison with the experimental results, not in the same horizontal scale [65].
In Figure 7.17 the dam front has advanced forward eroding the sediment surface. However the initial dune created by the dam break at $t = 0.25$ s is well defined in the experimental results. Also, the liquid motion creates a thick shear layer at the interface. These characteristics tend to be in agreement with the DP rather than the MC model.

In line with the previous discussion, Figure 7.18 and Figure 7.19 follow similar behaviour. However, at $t = 0.75$ s and $t = 1.0$ s the liquid free surface peak just after the scoured area of the initial dam break impact to the sediment bed is well defined in the experimental results.
Hence, the kinematics of the liquid have changed by the influence of the bed profile and scouring behaviour. As with before the agreement with the DP is closer.

![Experimental](image1)

(a)

![MC](image2)

(b)

![DP](image3)

(c)

Figure 7.19. Shear layer formation and shear layer velocity field for the Mohr-Coulomb (a) and the Drucker-Prager yield criterion at $t = 1.0$ s and qualitative comparison with the experimental results [65].

Since the qualitative results have demonstrated that the initial dam break impact at $t = 0.25$ s can influence the kinematics not only of the shear layer but the liquid kinematics a more detailed comparison is shown in Figure 7.20. Both yield models tend to be in good agreement with the experiment but at least in this experiment the DP appears to capture the experimental trend closely.
Partial summary

In this Section, a comparison of the suitability of two effective stress yield criteria models has been performed for static and dynamic cases with qualitative and quantitative comparisons with analytical solutions and experimental results. The surface yielding of the sediment was modelled by modifying the Newtonian constitutive equation to include yielding at the interface of the phases and a variable sediment viscosity for modelling the dynamics of the sediment. This approach is based on the infinitesimal rate of deformation of the material as a ratio between the yield stress and the sediment deformation.

In the cases examined small improvements were observed by the Drucker-Prager model. It has been shown that the bed profile and scour behaviour influences the kinematics not only of the sediment phase and the scour profile induced by the liquid, but also the liquid behaviour itself with the creation of dunes and scour holes changing the kinematics of the liquid considerably. Nevertheless, for a perfectly plastic frictional material prior knowledge of the behaviour of the material and the hydrodynamic application should be known.
7.2.2.2. Validation of Bingham constitutive models

Sediment block collapse

Numerical simulations are conducted to simulate experiments of Lube et al. [132] for granular flows. Lube et al. [132] investigated granular flow patterns of granular material block collapses under gravity. Such test cases serve validation purposes for the sediment single phase model and its rheological characteristics.

In the experiments, a vertical 3-D axisymmetric column of dry sand collapses under gravitational force with the flow behaviour depending on the aspect ratio of

\[ a_r = \frac{h_c}{r_c} \]

(7.8)

where \( h_c \) and \( r_c \) is the height and radius of the column respectively. Lube et al. [132] conducted a number of experiments for different aspect ratios and proposed a best fit equation based on the experimental results for \( a_r < 1.7 \),

\[ \frac{r_c - r_0}{r_c} = 1.24a \text{ for } a < 1.7 \]

(7.9)

where \( r_0 \) is the run-out distance measured from the centre of the column.

Herein, the more challenging low aspect ratio of \( a_r = 0.55 \) is chosen to validate the granular flow model as outlined by Chen et al. [38]. A vertical 2-D column with dimensions \( r_c = 0.1 \) m and \( h_c = 0.055 \) m has been chosen from the experimental results pool. The initial particle spacing was set to 0.001 resulting in 12 000 particles. The density of the simulated sand was 2600 kg/m\(^3\) with the Coulomb parameters set \( \phi = 30^\circ \) without cohesion (\( c = 0 \) Pa).

Since the granular material is dry, a Bingham model has been used and therefore the Herschel-Bulkley-Papanastasiou model was reduced to a Bingham model by setting the exponential growth of stress \( m \) to 0 and the power law index \( n \) to 1. To address the discontinuity associated with the Bingham model as \( II_d \to 0 \), a maximum yield stress is used with \( \tau_y = 2000 \) Pa which is representative of such flows [86, 195].

Figure 7.21 shows the accumulated \( \tau_y \) and Figure 7.22 the pressure field for the collapsing sand column. Notable are areas at the top of the column of the yield strength and pressure where accumulation of yield strength has occurred either by creeping or particle stacking similar to Section 7.2.1 since the sediment phase does not include the shifting algorithm.
Figure 7.21. Yield strength of the sediment at rest.

Figure 7.22. Pressure field after the soil column collapse.

Figure 7.23. Results reported from Chen et al. [38] for the soil column collapse case.

In addition results reported by to Chen et al. [38] using a non-associated flow rule are shown in Figure 7.23. The run-out distance comparison between the current and Chen et al. [38] model are in agreement to the equation of Lube et al. [132] predicting an run-out distance of
0.168 m which is the same as the numerical run-out distance. The profile of the experimental and numerical profile for the collapsing column of sediment is shown in Figure 7.24.

![Figure 7.24. Comparison of experimental [132] and SPH numerical profile of the collapsing sand column.](image)

The numerical profile is in good agreement with the experimental data with a deviation near the boundary. The divergence from the experimental results near the boundary is mainly due to the dynamic boundary conditions used in this work and is being discussed further in Section 8. However, small departures are also observed at the top of the column. Nevertheless, the agreement is satisfactory.

**Sediment Dam Break**

Bui et al. [26] conducted 2-D dam break experiments with aluminium bars to validate the numerical solution of a non-associative flow rule model based on the Drucker-Prager criterion and reported results on the profile of the dam after collapse but most importantly the failure area of the sediment dam which was unavailable in the sediment block collapse. In the experimental setup aluminium bars with a diameter of 0.001 m and 0.0015 m and length of 0.05 m where used to simulate 2-D conditions. In the numerical experiment, equal number of experimental aluminium bars and particles has been used with an initial particle spacing of 0.002 m resulting in 5000 particles resulting in one-to-one simulation. The dam dimensions were \( L = 0.2 \) m and \( H = 0.1 \) m with a density of \( \rho = 2650 \) kg/m\(^3\). The friction angle was to be \( \phi = 19.8^\circ \) on a non-cohesive material.
Figure 7.25. Dam break comparison between: (a) the experimental results of Bui et al. [26], (b) numerical results of Bui et al. [26] with the yield surface and (c) results of the current numerical model and comparison of the experimental profile and yielded surface of the aluminium bars, black dots denote free-surface and red dots yield surface profile.
Figure 7.25 shows a comparison between the experimental and numerical results of Bui et al. [26] and the results of the current model including the surface profile and yielded area comparison with the experimental results. The agreement of the dam break surface is satisfactory with the exemption of the run-off area of the dam break toe. Also, the yield surface shows good agreement with the experimental data and the numerical results of Bui et al. [26]. The issue on the run-off toe was identified in the collapsing sand box experiment and is associated with poor boundary conditions. This is clearly identified in the pressure field of Figure 7.26. The toe front of the dam seems to have high pressure at the front with spurious pressure field near the run-off area. Finer resolution simulations reduced the error but the spurious pressure field near the run-off area was still present.

Similar issues have been identified by other researchers using the dynamic boundary conditions of DualSPHysics in multi-phase simulations [144].

![Pressure field](image)

Figure 7.26. Pressure field of the collapsing dam break, note the poor pressure prediction at the toe front, black dots denote free-surface and red dots yield surface profile.

Concluding, the comparison of the numerical experiment with the available results from Bui et al. [26] were in close agreement for the dam break surface and the yielded region. Small departures have been observed on the dam break toe which is mainly associated with the boundary conditions used in DualSPHysics.
Erodible dam break

The erodible dam break of Section 7.2.2.1 is re-examined using the multi-phase model implementation of Section 5.3 but using the Herschel-Bulkley-Papanastasiou constitutive equation and suspension model.

The geometrical characteristics and particle spacing is identical to the erodible dam break of Section 7.2.2.1. The density ratio between the two phases is $\rho_s = 1.54 \rho_l$ as previously but the viscosity and Coulomb parameters slightly modified to allow a direct comparison with results obtained by the experiments and Ulrich et al. [195]. Hence, the sediment and liquid viscosity was set to $\mu_s = 500 \text{ Pa s}$ and $\mu_l = 10^{-3} \text{ Pa s}$ respectively and the sediment Coulomb parameters were set to $c = 0 \text{ Pa}$ and $\phi = 31^\circ$. The Herschel-Bulkley-Papanastasiou parameters of exponential growth of stress and power law index were set to $m = 10$ and $n = 0.4$ respectively.

Figure 7.27 to Figure 7.30 show a qualitative comparison of the experimental profile and the numerical results. Also, a quantitative comparison has been performed using the experimental results, the results reported by Ulrich et al. [195] and the current model for the profiles of the liquid and sediment. The main difference with the Ulrich et al. [195] model is the treatment of the sediment phase by different yield criteria and constitutive equation (see Section 2.7.2).

![Figure 7.27](image1.png)

![Figure 7.28](image2.png)

![Figure 7.29](image3.png)

Figure 7.27. Qualitative comparison of (a) experimental [65] and (b) current numerical results and (c) comparison of liquid-sediment profiles of the experiments, numerical results of Ulrich et al. [195] and current model at $t = 0.25 \text{ s.}$
Figure 7.28. Qualitative comparison of (a) experimental [65] and (b) current numerical results and (c) comparison of liquid-sediment profiles of the experiments, numerical results of Ulrich et al. [195] and current model at $t = 0.50$ s.

Figure 7.29. Qualitative comparison of (a) experimental [65] and (b) current numerical results and (c) comparison of liquid-sediment profiles of the experiments, numerical results of Ulrich et al. [195] and current model at $t = 0.75$ s.
Figure 7.27 shows the liquid and sediment profile at $t = 0.25$ s which follows the initial dam release. The profile of the experiment seems to be in good agreement to the numerical profile and follows similar trend at the liquid surface and the interface between the liquid and the soil mixture. As small departure is visible at the toe front of the dam were the numerical model run off distance is marginally forward of the experimental. Nevertheless, comparing the numerical profile with the numerical results of Ulrich et al. [195] demonstrates that the model of Ulrich et al. under predict the scouring profile at $x = 0.0$ m and the creation of a dune at $x = 0.1$ m considerably and therefore the liquid surface peak around that area of the dam toe front.

This underprediction of the liquid surface peak and scour profile is also visible in Figure 7.28 where Ulrich et al. reported minimal scouring at the interface. However, even if the current model follows the scouring profile trend of the experiments at $t = 0.50$ s it over predicts the interfacial profile. However, the liquid surface is well represented.

This over prediction of scouring is present at $t = 0.75$ s and $t = 1.00$ s in Figure 7.29 and Figure 7.30, moreover the current numerical profile is in better agreement to the numerical model of Ulrich et al.
Concluding, the Ulrich et al. [195] and the current numerical model compared herein performed reasonably well reproducing closely the hydrodynamics and scouring process. However, the profile trend of the current model appears to follow the liquid surface and interface profile closer. The scouring profile was slightly overestimated in the current model at $t = 0.5$ s and it is believed to be linked with the shear thinning parameter of the Herschel-Bulkley-Papanastasiou model. The tuning parameters of constitutive equations such as the current, the GVF or more advanced models has been reported to be a drawback since it is difficult to [35, 38, 176] determine experimentally.

Note that the issues identified earlier with the boundary conditions employed in DualSPHysics were not present in this test case since the kinematics of the flow were mainly associated with the interface.

**Partial conclusions**

In Section 7.2.2.2, validation cases have been presented for dry and saturated sediment under gravity, pressure and liquid induced flow. The sediment block collapse showed good agreement with the experimental profile and similar behaviour to other numerical models. However, accumulated yield strength was present in some streamlines of the soil. These streamlines were symmetrical similar to the streamlines present in the liquid phase when shifting was not employed. Therefore a second test case has been presented. The soil dam break showed good agreement with the profile of the experiments but most importantly the agreement of the numerical yield surface and the experimental was satisfactory. The erodible dam break was revisited using a more elaborate constitutive equation. The agreement with the experiments was reasonable and the model showed some improvements over similar numerical models. However, at some points some over prediction of the scour profile occurred.

Next, a liquid-sediment 3-D case is presented. The 3-D case has served as a benchmark case for validating shallow water equation models in the past and to the authors best knowledge this is the first time this test case has been performed in SPH.
7.3. 3-D validation case

7.3.1. 3-D erodible dam break

In this Section, the numerical results for a 3-D validation case are presented. The experimental test case of Soares-Frazão et al. [186] provides validation data for numerical models for dam-break simulations over mobile beds with fast transient flows involving sediment transport. Initially, the test case was set as a benchmark blind test to the scientific community with twelve modelling teams participating.

Figure 7.31. Schematic of the 3-D dam break experiment.

In the experiments a 27 m long flume is used where the breached dam is represented by two impervious blocks with a 1 m wide gate located between the blocks. Figure 7.31 shows a schematic of the experiment. The height of the sediment was 0.085 m located 1.5 m before the gate and extended to over 9.5 m as shown by the hatched area of Figure 7.31. The sediment had a uniform coarseness with a sediment to fluid density ratio of 2.63 and a porosity of \( n_r = 0.42 \). The fluid height in the reservoir was 0.470 m for the current experiment case.

Two measuring points US1 and US6 were used to measure the water levels and three bed profiles were taken at \( y_1 \), \( y_2 \) and \( y_3 \) at the end of the simulation at \( t = 20 \) s (Figure 7.31).

In the numerical model the particle spacing was set to 0.005 m resulting in 4 million particles of which 701132 were boundary particles. The initial density of the fluid and sediment was
set to hydrostatic and lithostatic conditions, respectively. The $\delta$-SPH parameter for this simulation was set to 0.1 as recommended in [145]. The fluid dynamic viscosity was 0.001 Pa s and sediment viscosity was set to 150 Pa s with the HBP $m$ and $n$ parameter set to 100 and 1.8 respectively. The value for the exponential growth $m$ parameter value was chosen to approximate a Bingham model as closely as possible with a minimal pseudo-Newtonian region and the power-law exponent $n$ to resemble shear thinning materials as shown in Figure 5.8.

Finally a small amount of cohesion was given to the sediment phase of $c = 100$ Pa to stabilise the interface and control the scouring near the dam gate. Next, the water level height and sediment profiles are presented against the experimental data for the aforementioned control points.

### 7.3.1.1. Sediment bed profiles

The sediment profile in comparison with the experimental data is shown in Figure 7.32 for three different cross-sections of the sediment bed at locations $y_1$, $y_2$ and $y_3$. The numerical results are compared with four different runs (labelled as b1, b2, b3 and b4) of the experiment as reported by Soares-Frazão et al. [186] with the SPH data superimposed on the experimental data. The data from runs b1 –b3 show the variability in the experimental data. The bed profile at $y_1$ shows satisfactory agreement with the experimental results for most of the length of the bed. Some deviations from the experiment are notable specifically near the dam break gate around $x = 0.5$ m to $x = 1.5$ m. Nevertheless, looking at the experimental results there is a disagreement on the repeatability of the experimental results with some of the runs having lower scouring at the front with a peak for runs $b$ and $d$ whereas the numerical results are in better agreement with runs $a$ and $c$.

Also a small deviation is observed at $x = 4.0$ m where the numerical model over predicts the scouring region. At $y_2$, the agreement is marginally better from $y_1$ with only small deviations near the wall at $x = 0.5$ m which were expected due to the boundary conditions implemented in DualSPHysics that exhibit a sticking behaviour near the walls. Finally, the $y_3$ bed profile shows similar problems near the wall over predicting the sediment height. Also, the sediment peak is slightly underestimated with a small delay on the location of the peak.
Figure 7.32. Repeatability of the bed profiles at locations (a) $y_1$, (b) $y_2$ and (c) $y_3$ of the experiment and comparison with the numerical results.
However, due to the complexity of the 3-D dam break case the sediment profiles are satisfactory since the hydrodynamics of the fluid are very complex especially at the gate with a rarefaction wave and an initial hydraulic jump. Unfortunately, results of the upstream flow or near the gate are not reported in Soares-Frazão et al. [186]. A snapshot of the velocities of the sediment from the numerical experiment is shown in Figure 7.33 and the profile of the bed is shown in Figure 7.34.

Figure 7.33. Velocity magnitude profile of the bed at $t = 20$ s.

Figure 7.34. Height profile of the sediment at $t = 20$ s.
7.3.1.2. **Water level measurements**

The hydrodynamics of the flow are linked to the sediment scour mechanisms. In this section, two water level probes locations are used to measure the numerical water levels and compare with the experimental results of the 3-D dam break. The experimental profiles for gauge US1 and US6 shown in Figure 7.31 are compared with the numerical run.

![Graph](image)

**Figure 7.35.** Repeatability of the water level measurements of the experiment for gauge US1 and US6 and comparison with the numerical results.
The water height levels of gauge US1 which is located near the gate shows reasonable agreement with the reported experimental results with a small over prediction at the end of the simulation. Similar results are shown for gauge US6 with the exception of a sudden drop in water level at \( t = 3.0 \) s. We suspect the water level deviation at this point is an error in numerical sampling and is under further investigation. Another explanation might be an absence of fluid particles in the sampling control volume since we are using sparse sampling times of 1 s. Both graphs show slightly over predicted water heights particularly at location US6. This is in agreement with the sediment profiles which show a small bed thickness over prediction downstream.

Concluding, the results from the numerical experiment of a 3-D dam break have been compared with a benchmark dam break case of Soares-Frazão et al. [186]. To the authors best knowledge this is the first time this test case has been performed in SPH mainly due to the large domain and therefore the high computational cost. The 3-D erodible dam break took 14 days on a Tesla K20 GPU card for 4 million particles.

The 3-D dam break bed profiles of the sediment located at the bottom of the tank were satisfactory reproduced in this numerical experiment with only small deviations near the gate and a small over prediction downstream that might be accounted for by the departure in the dynamics of the fluid. Also the water level profiles at two discrete locations have been presented with reasonable agreement to the experimental results.

### 7.4. Concluding Remarks

In this Chapter a number of test cases have been presented to validate and verify the capability of the model presented in Chapter 5 using the GPU implementation of Chapter 6. The test cases have been chosen for their simplicity and available analytical or experimental data. In addition, some of the validation cases have been used in the past by other SPH practitioners and therefore a direct comparison of the numerical models that reveal the advantages and disadvantages of each model can be easily identified.

Both phases have been validated targeting the proposed modifications such as the liquid phase and the droplet test case which yield interesting conclusions on the use of \( \delta \)-SPh and the shifting algorithm. The pressure field of the droplet using the shifting algorithm changed the pressure of the droplet at impact significantly and was in agreement in comparison to other numerical methods (Volume-of-Fluid).
The sediment phase was validated using a single and two-phase flow under gravity and pressure fields and liquid-driven scour. Two different approaches were demonstrated using different constitutive equations. The first used the ratio of the yield stress and the strain rate of the sediment mixture which is a traditional approach in SPH, whereas the second applied a constitutive equation proposed by Herschel-Bulkley-Papanastasiou. The HBP model resembles a Bingham model but allows for control on the stress growth and low strain states and can be tuned to model shear thinning or thickening materials. This Bingham model was applied to a 2-D and 3-D validation case with significant agreement to the available data.

Observed disadvantages are mainly inherent not only to the model but the SPH method itself. In both phases a discrepancy near the boundary has been observed. Near the wall boundary departures from reference data were obvious. The wall boundary conditions used in DualSPHysics have been known to cause large separation regions with unphysical pressure fields. In Chapter 8, presents a new improved wall boundary conditions with considerable success in 2-D and 3-D. On the other hand the two-phase model showed slight over prediction of the scouring characteristics in the 2-D and 3-D cases. However, the hydrodynamics of the flow dictate the amount of scouring and the resulting interfacial profile that may be challenging to predict at the interface. Nevertheless, the free surface of the liquid and scouring profile was in good agreement with the available reference data.
Chapter 8

8. A new wall boundary condition

8.1. Introduction

Due to the intrinsic nature of kernel based interpolation and to the Lagrangian approach, imposing boundary conditions in SPH is still an open problem. The approach proposed by Monaghan [147, 152] is the repulsive force method; where the wall is described by particles which exert a repulsive short-range force similar to a Leonard-Jones potential force on fluid particles.

Mirror or ghost particles as introduced by Randles and Libersky [174] is another widely used way to describe boundaries in SPH [45, 139]. Kulasegaram et al. [106] proposed a variant of this method which introduces an additional term in the momentum equation in order to mimic the effect of the wall. This technique eventually uses an empirical function to approximate the force originating from variational principles. The idea was further developed in [17, 51, 58]. These methods have the advantage of restoring zero-consistency in the SPH interpolation but the discretization of complex 3-D geometries and/or multi-phase flows is not straightforward. In general, the repulsive force method is more flexible because it can be used to describe complex moving boundaries, but it can introduce a non-physical pressure oscillation and it does not reduce the effect of kernel truncation near the wall.

Ferrari et al. [59] proposed a local point symmetry (as opposed to ghost particles) method, called the Virtual Boundary Particle (VBP) technique which is able to discretize arbitrarily complex geometries without introducing empirical forces. Recently the method was further enhanced and applied to shallow water equations (SWE) [197]. A more elaborate literature review is given in Section 2.5.5.

In this thesis, the aforementioned VBP wall boundary method is further enhanced and applied to the Navier-Stokes in order to ensure approximate zeroth and first-order consistency in presence of arbitrarily complex boundaries using the WCSPH approach in 2-D and lately in 3-D. However, the method can be applied to the ISPH methodology.
In this work, the zeroth and first-order consistency refers to the ability of the scheme to reproduce a zeroth order polynomial and a linear function, respectively by the SPH interpolation method. The term “approximate” refers to the discretized form of the SPH interpolation as the particle spacing $dx \to 0$.

This Chapter firstly addresses the issue of particle inconsistency and kernel truncation near the boundary. Next, the extended modified virtual boundary particle (eMVBP) wall treatment is presented for the Navier-Stokes equations in 2-D followed by the numerical results for static and dynamic cases. However, efficiency and GPU parallelisation is essential in 3-D simulations. The latter method is extended to 3-D to treat arbitrarily complex geometries by describing the solid wall using triangles to maximize the efficiency and GPU parallelization. Approximate zeroth and first order consistency are ensured by using a fully uniform fictitious particle stencil.

### 8.2. Particle inconsistency in SPH

#### 8.2.1. Kernel particle consistency

Numerical schemes should be able to reproduce the corresponding physical equations of the continuum domain in a discrete form. SPH achieves that using a smoothing kernel function (smoothing kernel or kernel) which dictates the connectivity of the surrounding nodes and the region of influence for each interpolated particle.

For a scalar function $f$ to be reproduced exactly, $f$ must be smooth and continuous in $\Omega$ and the smoothing length $h \to 0$ resulting in a Dirac delta function

$$f(x) = \int_{\Omega} f(x') \delta(x - x', h) dx',$$

but since the smoothing length is finite, the integral representation is only an approximation consistent to the $n^{th}$ order. The accuracy of the approximation can be demonstrated by applying a Taylor series expansion of $f(x')$ around $x$ of Equation (8.1)

$$f(x) = \sum_{k=0}^{n} \frac{(-1)^k h^k f^{(k)}(x)}{k!} \int_{\Omega} \left( \frac{x - x'}{h} \right)^{(k)} W(x - x', h) dx' + O \left( r \left( \frac{x - x'}{h} \right) \right),$$

with $k$ the order of the derivative. For a function $f$ to be approximated to $n^{th}$ order accuracy the following moments must hold [11, 126],

176
\[
\begin{align*}
M_0 &= \int_\Omega W(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 1 \\
M_1 &= \int_\Omega (\mathbf{x} - \mathbf{x}')W(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 0 \\
M_2 &= \int_\Omega (\mathbf{x} - \mathbf{x}')^2W(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 0, \\
&\vdots \\
M_n &= \int_\Omega (\mathbf{x} - \mathbf{x}')^nW(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 0 
\end{align*}
\] (8.3)

assuming that at the support edge the \((k-1)\)th derivative of the kernel vanishes \(W^{(k-1)}(\mathbf{x} - \mathbf{x}', h)|_S = 0\) where subscript \(S\) denotes the surface of the support. These expressions form the fundamental conditions for the integral approximation, i.e. the first expression of Equation (8.3) satisfies a zeroth order polynomial \(f(\mathbf{x}) = c\) and the second expression of Equation (8.3) a linear function \(f(\mathbf{x}) = a\mathbf{x} + c\) exactly. This procedure can be similarly repeated for the first derivative:

\[
\begin{align*}
M'_0 &= \int_\Omega W'(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 0 \\
M'_1 &= \int_\Omega (\mathbf{x} - \mathbf{x}')W'(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 1 \\
M'_2 &= \int_\Omega (\mathbf{x} - \mathbf{x}')^2W'(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 0, \\
&\vdots \\
M'_n &= \int_\Omega (\mathbf{x} - \mathbf{x}')^nW'(\mathbf{x} - \mathbf{x}', h)d\mathbf{x}' = 0 
\end{align*}
\] (8.4)

In addition, to these conditions the kernel function should also vanish over the surface \(S\) of its region of influence \(\Omega\):

\[W^{(k-1)}(\mathbf{x} - \mathbf{x}', h)|_S = 0.\] (8.5)

The purpose of investigating the gradients moments is to introduce a numerical measure of the scheme to correctly predict gradients (first-order in this paper) without the computation of the gradients being complicated by physics of the problem or equation sets.

Typically in SPH, only the first derivative is used when dealing with the Navier-Stokes equations since the second derivative is either expressed as the derivative of the first [123] or by using a combination of a central differencing scheme for the first derivative and an SPH interpolation for the second [157].

In a continuous form, in the interior domain away from boundaries and for a uniform distributed stencil of particles both the zeroth and first order moments of Equations (8.3) and (11) are satisfied since the support is complete, where the kernel symmetry and
\( W(\mathbf{x} - \mathbf{x}', h) = W(\mathbf{x} + \mathbf{x}', h) \) and kernel gradient asymmetry \( \nabla_x W(\mathbf{x} - \mathbf{x}', h) = -\nabla_x W(\mathbf{x} - \mathbf{x}', h) \) properties of the kernel, also hold. In addition, \( W^{(k)}(\mathbf{x} - \mathbf{x}', h) |_{x} = 0 \) is satisfied. Therefore, constant and linear functions can be exactly reproduced and the SPH method is said to have \( C^1 \) consistency. In discrete form the zeroth and first order moments of the kernel function are only approximately satisfied.

\[
\begin{align*}
M_0 &= \sum_{j} W(\mathbf{x} - \mathbf{x}_j, h) \frac{m_j}{\rho_j} \approx 1, \\
M_1 &= \sum_{j} (\mathbf{x} - \mathbf{x}_j) W(\mathbf{x} - \mathbf{x}_j, h) \frac{m_j}{\rho_j} \approx 0,
\end{align*}
\]

and similarly the moments for the derivative of the kernel can be written as

\[
\begin{align*}
M'_0 &= \sum_{j} \nabla_x W(\mathbf{x} - \mathbf{x}_j, h) \frac{m_j}{\rho_j} \approx 0, \\
M'_1 &= \sum_{j} (\mathbf{x} - \mathbf{x}_j) \nabla_x W(\mathbf{x} - \mathbf{x}_j, h) \frac{m_j}{\rho_j} \approx 1,
\end{align*}
\]

where \( N \) is the number of \( j \) neighbouring particles within the kernel support, \( m \) is mass and \( \rho \) is the density of a fluid particle. For a non-uniform distributed particle stencil, the discretization becomes exact when \( h \rightarrow 0 \) and \( N \rightarrow \infty \). The error of a finite \( h \) and \( N \) has been investigated by many researchers and is beyond the scope of this paper to further analyse, but in general it is assumed to be of the order of \( O( f (N)^{1/2} ) \) [204]. In this work we call approximate \( C^1 \) consistent that obtained for the discrete SPH interpolation with no kernel correction.

### 8.2.2. Inconsistency of the kernel near the boundary

Near the boundaries, considering SPH approximation in discrete form, the kernel support is truncated since Equations (8.6) and (8.7) are not satisfied, resulting in errors in the reproduction of constant and linear functions. Figure 8.1 illustrates the mechanism of a support domain truncated by a boundary in 1-D.
To restore approximate consistency near the boundary due to truncation for a uniform particle distribution Equation (8.6) is rewritten as a summation over interior fluid particles and particles that represent the boundary

\[
M_0 = \sum_{j \in F} W(x - x_j, h) \frac{m_i}{\rho_j} + \sum_{j \in B} W(x - x_j, h) \frac{m_i}{\rho_j} \\
M_1 = \sum_{j \in F} (x - x_j)W(x - x_j, h) \frac{m_i}{\rho_j} + \sum_{j \in B} (x - x_j)W(x - x_j, h) \frac{m_i}{\rho_j}
\]  
(8.8)

and similarly for the derivative, Equation (8.7) becomes

\[
M_0' = \sum_{j \in F} \nabla_x W(x - x_j, h) \frac{m_i}{\rho_j} + \sum_{j \in B} \nabla_x W(x - x_j, h) \frac{m_i}{\rho_j} \\
M_1' = \sum_{j \in F} (x - x_j)\nabla_x W(x - x_j, h) \frac{m_i}{\rho_j} + \sum_{j \in B} (x - x_j)\nabla_x W(x - x_j, h) \frac{m_i}{\rho_j}
\]  
(8.9)

where the \( F_p \) represents the set of \( N_f \) interior fluid particles and \( B_p \) represents the set of \( N_b \) particles representing the boundary. Clearly, for a \( C^1 \) consistent approximation the interior and the boundary particles summation are needed near the wall, with the \( N_b \) summation providing the truncated support of the wall.

The Equation of conservation of mass (4.7) in discrete form near the wall boundary can now be written as

\[
\frac{d\rho_j}{dt} = \sum_{j \in F} m_i u_{ij} \cdot \nabla W_{ij} + \sum_{j \in B} m_i u_{ij} \cdot \nabla W_{ij}.
\]  
(8.10)
With the as-yet unspecified boundary particles, the kernel support can be considered complete, hence the anti-symmetric property of the kernel is not violated and therefore, conditions of Equation (8.8) and (8.9) are satisfied. Note, that the conservation of mass for a volume \( V \) still holds and integration of the continuity equation (8.10) yields

\[
\frac{dm}{dt} = 0, \tag{8.11}
\]

at any control volume locally and therefore, the global mass of the system remains unchanged for a closed system.

Following the same approach, the inviscid momentum Equation (4.17) can be written as

\[
\frac{du}{dt} = -\sum_{j \in F} m_j \left( \frac{P_i + P_j}{\rho_i \rho_j} \right) \nabla W_{ij} = -\sum_{j \in B} m_j \left( \frac{P_i + P_j}{\rho_i \rho_j} \right) \nabla W_{ij} + \mathbf{g}_i. \tag{8.12}
\]

Vaughan et al. [204] has demonstrated that the momentum equation can be recovered from the inviscid discretized form

\[
\frac{du}{dt} = -\sum_{j} m_j \left( \frac{P_i + P_j}{\rho_i \rho_j} \right) \nabla W_{ij} + \mathbf{g}_i, \tag{8.13}
\]

by performing a Taylor series expansion on \( P_j \) resulting in

\[
\frac{du}{dt} = -\frac{1}{\rho_i} \sum_{j} m_j \left( P_i + P_j + (x_j - x_i) \nabla P_j + ... \right) \nabla W_{ij} + \mathbf{g}_i. \tag{8.14}
\]

Equation (8.14) can be rewritten using the moments of the derivative of the kernel as

\[
\frac{du}{dt} = -\frac{\nabla P}{\rho_i} \mathbf{M}_1 + \frac{P}{\rho_i} \mathbf{M}_0' + O(f(N)) + \mathbf{g}_i. \tag{8.15}
\]

Since \( M_0' \approx 0 \) and \( M_1' \approx 1 \) when \( N = N_f + N_b \) the original momentum equation is recovered, demonstrating how the moments \( M_0' \) and \( M_1' \) are influencing the accuracy of the SPH interpolation of the derivative of the kernel. However, if moments are truncated, Equation (8.15) is not correctly approximated and the original form is not recovered.

In summary, the particle inconsistency manifested from the truncated kernels near the boundary in discrete form can be recovered by adding the truncated support of the kernel in uniform particle distributions resulting in approximate \( C^1 \) consistency. As demonstrated both mass and momentum is conserved while no additional terms are added to the SPH particle approximation. This is in contrast to the semi-analytical approaches of Ferrand et al. [58] and
Mayrhofer et al. [143]. A more detailed discussion for the boundary particles resulting in a larger pool of particles appearing within the truncated support in terms of the Navier-Stokes equations and the hydrodynamic correction is discussed in the next Section.

8.3. Wall Boundary conditions in 2-D

8.3.1. Existing Virtual boundary Particle (VBP) methods

The proposed wall boundary condition presented herein is an extension of the modified virtual boundary particle (MVBP) as described by Vacondio et al. [197] for shallow water equations. The method is based on a local point of symmetry of the boundary particles. In both the original VBP and MVBP methods walls are discretized using virtual particles which do not interact with fluid particles but are used only to create a set of fictitious particles for each fluid particle close to the walls.

As shown in Figure 8.2 (a), Ferrari et al., [59] proposed a wall boundary condition method, based on the local point of symmetry, by using virtual particles located on the wall boundary (solid black circles), that are used only for geometrical purposes (see below). An interior fluid particle \( h \) away from the wall produces a symmetric set of fictitious particles at a distance

\[
\mathbf{x}_k = 2\mathbf{x}_v - \mathbf{x}_i,
\]  

where \( k \) is the fictitious particle, \( v \) is the virtual boundary particle and \( i \) the interior fluid particle shown in Figure 8.2 (a). The virtual boundary particles are excluded from the SPH summation whereas each of the fictitious \( N_b \) particles (belonging to the set of boundary particles \( B \)) are included in Equations (8.10) and (8.12). The physical properties of fictitious particles \( k \) are associated with the fluid particle through

\[
\begin{align*}
  m_k &= m_i, \\
  \rho_k &= \rho_i, \\
  \mathbf{u}_k &= 2\mathbf{u}_v - \mathbf{u}_i
\end{align*}
\]  

(8.17)

However, the generation mechanism of Ferrari et al. [59] still has a partially truncated support and does not satisfy the Neumann condition at the boundary required to recover hydrostatic pressure (see discussion below). In Section 8.4 we will demonstrate that this leads to numerical instability.

Vacondio et al. [197], enhanced the generation mechanism with the MVBP technique to closely resemble the interior particles in uniform distribution from the truncated support by
including a second set of particles and therefore, reducing the error of the kernel moments of
Equation (8.6) and (8.7)

\[ \mathbf{x}_{k,1} = 2\mathbf{x}_v - \mathbf{x}_i \]
\[ \mathbf{x}_{k,2} = 4\mathbf{x}_v - \mathbf{x}_i \]

(8.18)

where the subscripts \( k,1 \) and \( k,2 \) denote the two sets of fictitious particles generated by the
local point of symmetry shown in Figure 8.2 (b).

Figure 8.2. Fictitious particle mechanism comparison using the (a) VBP and (b) MVBP for a straight
boundary.

In addition, it was identified that the spacing of the virtual particles should be distributed at a
spacing of \( \Delta x/2 \) (with \( \Delta x \) defined as the uniform particle separation). Furthermore, corners
need a special treatment for the generation mechanism to reconstruct the support of uniform
interior particles with the corner stencils for the VBP and MVBP shown in Figure 8.3 (a) and
(b), respectively. For the MVBP method, two virtual particles are added to the corner for
internal angles of \( \theta < 180^\circ \) as shown in Figure 8.3 (b). The resulting MVBP stencil was
demonstrated to be superior to the original approach (see [197]).

The disadvantage of the MVBP generation mechanism comes from the shortage of particles
generated using the second Equation of (8.18). In a case where \( h = 1.3\Delta x \) (as commonly used
in SPH and throughout this paper), an interior fluid particle located at a distance \( \Delta x \) or more
away from the wall in a uniform arrangement of fluid particles, the MVBP is not able to guarantee that the stencil for that particle is identical to a stencil for particle in a uniform arrangement away from the boundaries (see Figure 8.2 (b)). This deficiency is directly addressed by the eMVBP method as demonstrated in Figure 8.4 (a) and (b), where the red solid circles denote the extra two fictitious particles generated by the method proposed herein in Section 8.3.2.

Also, the extra virtual particles (black solid lines) added to the internal corners of $\theta < 180^\circ$ for the MVBP deem the method difficult to generalise in large complex arbitrary domains. In this work, a more general mechanism is used that is shown in Figure 8.5 where the red circles in Figure 8.5 (b) represent the new particles to be generated by the eMVBP approach.

![Figure 8.3. Fictitious particle mechanism comparison using the (a) VBP and (b) MVBP on a 90° corner.](image)
8.3.2. Generation of fictitious particles

The new method proposed herein is an extension of the modified virtual boundary particle approach (eMVBP) that addresses the issues of the VBP and MVBP in the framework of the Navier-Stokes equations in such manner that consistency is significantly improved near the
boundary. Three different modifications are proposed to ensure a uniform particle stencil and complete support for any particle near the boundary:

(i) In particular, different from the VBP and MVBP, the complete support is ensured not only for particles within a distance $h$ from the boundary but also for all fluid particles whose smoothing kernel overlaps with the boundary. This has not been addressed by Ferrari et al. [59] and Vacondio et al. [197].

(ii) For a non-uniform fluid particle distribution the fictitious particles are generated with uniform stencil unlike the previous authors. This maintains a uniform shear stress on a particle moving parallel to the wall in a steady flow.

(iii) Finally, the particle properties (density, mass and velocity components) are defined using local point of symmetry to satisfy the hydrostatic conditions and the Neumann boundary condition on pressure.

In the following description, we refer to Figure 8.6 which shows the different possible positions where an interior fluid particle could be located generate fictitious particles. The distance of the fluid particle is denoted by $|x_i - x_v|$. The method comprises of two generation zones:

(i) **Particles with** $2h > |x_i - x_v| > h$

The first generation zone concerns interior fluid particles which are at a distance between $2h$ and $h$ from the wall, i.e. $2h > |x_i - x_v| > h$, shown in Figure 8.6 (b) and (c). These are interior fluid particles located at a distance $h$ away from the boundary where part of their support is truncated by the wall.

For example, in the uniform particle arrangement of Figure 8.4 (b), these are the interior fluid particles in the second row adjacent to the boundary wall. For a fluid particle with a distance less than $2h$ but greater than $h$ from the wall, a fictitious particle is generated at the edge of the kernel support at a distance $n_{smt}\Delta x$ (recalling that $\Delta x$ is the particle separation) with the fictitious particle position given by

$$x_k = x_v - (n_{smt}\Delta x n - x_{iw}),$$

(8.19)

where $n_{smt} = \text{int}(2h / \Delta x)$ is the number of fictitious particles that fit within the support radius (for example for the Wendland kernel adopted herein with $h = 1.3\Delta x$, $n_{smt} = 2$) and int(…)
gives the integer value of the argument. This ensures that each virtual particle generates a sufficient set of fictitious particles to complete the support for any smoothing length.

As the interior particle distance to the wall is reduced the fictitious particle distance to the boundary is increasing until it reaches $h$ and is discarded whereupon the second generation zone will replace the fictitious particle with the mechanism of Equation (8.20), ensuring continuity and uniformity in the fictitious particle continuity.

![Diagram](image)

Figure 8.6. Generation mechanism snapshots as a fluid particle shown in a hatched circle (a) approaches the solid wall. The first generation mechanism is shown in (b) and (c) denoted with a red solid circle and the second generation zone in (d) and (e) denoted with a blue solid circle.

(ii) Particles with $h \geq |x_i - x_v|$  

The second generation zone is applied to particles within a distance less or equal to $h$ to the wall boundary as with the MVBP, Figure 8.6 (d) and (e). Equation (8.18) is modified as follows
\[
\begin{align*}
\mathbf{x}_{k,1} &= 2\mathbf{x}_v - \mathbf{x}_i \\
\mathbf{x}_{k,2} &= 4\mathbf{x}_v - 3\mathbf{x}_i \\
&\vdots
\mathbf{x}_{k,n_{	ext{smt}}} &= 2n_{\text{smt}}\mathbf{x}_v - (2n_{\text{smt}} - 1)\mathbf{x}_i.
\end{align*}
\] (8.20)

### 8.3.3. Virtual particles shifting

The generated fictitious particles must always be uniformly distributed with respect to the interior fluid particle since \( \sum_j (\mathbf{x} - \mathbf{x}_j)\nabla W(\mathbf{x} - \mathbf{x}_j, h) \frac{m_j}{\rho_j} \neq 1 \) in a discrete domain, as discussed in Section 8.2. When the interior fluid particle interacts with the virtual boundary particles a search algorithm scans the virtual particles within the support so that

\[ dr = \min((\mathbf{x}_i - \mathbf{x}_v) \cdot \mathbf{\tau}) \]

where \( \mathbf{\tau} \) is the tangent vector of the virtual particle. The virtual particles are then shifted by the distance \( dr \) along the tangent line (which coincides with the wall boundary line) so that the new generated stencil is uniformly distributed with respect to the fluid particle as shown in Figure 8.7.

![Virtual particle shifting mechanism](image)

**Figure 8.7.** Virtual particle shifting mechanism to achieve uniform stencil for the (a) MVBP in comparison with the (b) eMVBP.

### 8.3.4. Generalisation for complex geometries

With complex geometries, such as internal and external corners and curves, the method requires a generalisation. A straightforward method to generalise the generation mechanism is to use new temporary reference system, where the \( z' \) axis is defined by the normal unit
vector pointing inside the fluid domain, and the $x'$ axis is at a tangent to the wall. This procedure requires the normal and tangent of the interacting virtual particle to be known and the normal points to the interior of the fluid domain (or all virtual particles normal follow the same notion). A rotation matrix $R$ is used to rotate the global axis by an angle $\theta$ defined between the global axis and the unit tangent $\tau$ of the virtual particle

$$X = xR,$$

where $X$ is the position of the fluid particle in the new reference system. By performing this relatively cheap operation for the position and velocity of the fluid and virtual particle, complex geometries can be treated readily. When the necessary geometrical operations have been performed (such as virtual particle shifting along the boundary tangent line, fictitious particle generation and velocity mirroring) the coordinates are rotated back to their original angle $x = X R^T$ and the global coordinates are used in the evaluation of Equations (8.10) and (8.12). Three examples are shown in Figure 8.8 for planes at different internal angles.

![Diagram](Image)

Figure 8.8. Generalisation for complex geometries using a rotation matrix, 3 cases of rotation according to the orientation of the boundary (a) 0°, (b) 45° and (c) 90°.

### 8.3.5. Fictitious particle flow properties in local point of symmetry

Since we are dealing with the Navier-Stokes equations, enforcing boundary conditions for the velocity and pressure of the fluid at the boundary is necessary. These boundary conditions are applied through the fictitious particles properties. In line with Ferrari *et al.* [59] and Vacondio *et al.* [197] the mass of the fictitious particle is

$$m_k = m_i,$$

which satisfies Equations (8.10) and (8.11). Since the flow is hydrostatic at rest, the density and pressure should be able to recover the hydrostatic pressure at the boundary. Therefore,
the density $\rho$ and pressure $P$ of the fictitious particles using the equation of state (Tait’s in this paper) are calculated as

$$\rho_k = \rho_i + \rho_0 \left( \frac{P_0 g z_{ik}}{B} + 1 - 1 \right),$$

$$P_k = P_i + \rho_0 g z_{ik},$$

where $z_{ik} = z_i - z_k$ and $B$ refers to the $c_0^2 \rho_0 / \gamma$ ratio of the Tait’s equation of state. These flow properties resemble hydrostatic conditions and should satisfy the Cauchy boundary condition [207]. Indeed, at the boundary, since pressure is corrected hydrostatically

$$P_k = \rho_0 g h,$$

where $H$ is the water depth. Equation (8.24) is the Dirichlet boundary condition for the pressure (if only non-accelerated boundaries are considered) and

$$\frac{\partial P}{\partial n} = \rho_0 g \cdot n,$$

which satisfies the Neumann condition required for the Cauchy condition. Note that as discussed in Section 8.2.1, the error in the SPH interpolation generated by non-uniform fluid particle distribution is not addressed in this work. Also, regardless of the position of the fluid particles, the fictitious particles generated for a given fluid particle are always distributed over a regular stencil. Liu and Liu [126] have demonstrated clearly that a regular stencil reduces the SPH interpolation error near the boundary as only a uniform particle distribution can satisfy the first moment of the kernel and its derivative.

The velocity field in the fictitious particles is assigned according to Takeda et al. [192] method

$$\mathbf{u}_k = (\mathbf{u}_v - \mathbf{u}_i) \frac{||\mathbf{x}_{ik}||}{||\mathbf{x}_{ii}||} + \mathbf{u}_v,$$

where subscript $v$ denotes the virtual particles and $k$ denotes the fictitious particles. This satisfies the impermeability condition $\mathbf{u} \cdot \mathbf{n} = 0$ necessary to conserve mass. Therefore, the truncated boundary correction imposes hydrostatic conditions for uniform and non-uniform interior particle distributions since each interior particle interacting with the virtual particle will in turn have hydrostatic conditions locally imposed by the truncated area.
The mirroring procedure described previously addresses the error arising from truncating the support near the boundaries. By applying hydrostatic conditions to the fictitious particles the hydrostatic pressure can also be correctly simulated at the boundary satisfying the Cauchy boundary condition. The velocity mirroring of Takeda et al. [192] guarantees a non-permeable wall with consistent first order differential operators [135].

8.4. Numerical results

8.4.1. Still water case

To evaluate the moments of the kernel and its derivative and inspect the pressure near the wall, the case of still water in a tank is an ideal case since the geometry contains two 90° internal angles and pressure discrepancies can be easily detected. A container with length and height equal to 4 m contains water of height $H = 1.95$ m. The particle spacing was set to $\Delta x = 0.1$ m resulting in 1041 fluid particles. Artificial viscosity with the free parameter $\alpha = 0.1$ is used in this case. The value of alpha has chosen in order to highlight the differences between the three boundary condition methods herein considered. The Shepard filter is used every 30 time steps and the speed of sound is assumed equal to $c_0 = 80$ m/s. At the beginning of the simulation the pressure is assumed hydrostatic, and the water is at rest. Simulations up to 20 seconds were performed.

First, the ability of the eMVBP to reproduce the still water conditions is examined. Figure 8.9 shows the hydrostatic pressure for the first time step at the vertical cross-section in the middle of the domain at $x = 2$ m. The first time step was chosen because the interior particle distribution is uniform. As stated in Section 8.3.5 the eMVBP approach should reproduce hydrostatic conditions for a uniform particle distribution. Indeed, Figure 8.9 demonstrates very good agreement with the analytical pressure whereas non-negligible errors are generated with both the VBP and MVBP methods.

Figure 8.10 shows the convergence behaviour for the eMVBP using the velocity $L_2$ error norm [197] in comparison to first and second order convergence for the still water case with satisfactory results.
Figure 8.9. Still water: hydrostatic pressure after the first time step at a vertical cross-section in the middle of the domain \((x = 2.0 \text{ m})\) against the analytical solution for all three methods.

Figure 8.10. Still water case: velocity \(L_2\) error norm convergence.

Now we turn our attention to the moments of the kernel and its derivative, as defined in Equations (8.8) and (8.9).
In Figure 8.11, the zeroth and first order moments for the kernel and its derivative in $z$ direction are plotted along a cross section at $x = 2.0$ m. Since our approach only treats the boundary, we will ignore the inner and free surface of the interior domain and hence plots are generally limited to 1 m height. Moreover the $x$ direction is ignored, as the particle distribution is uniform and symmetric and does not influence the moments. We recall that ideally the zeroth order moment has value of unity, the first moment should be zero while their gradients should be zero and one, respectively.

Figure 8.11. Still water for the first time step: (a) zeroth and (b) first moment of the kernel, (c) zeroth and (d) first moment of the kernel derivative in $z$ direction.

Figure 8.11 (a) shows considerable improvement for the eMVBP over its predecessors with a near constant zeroth moment. The drawbacks of the MVBP and VBP can be clearly seen. Neither the VBP nor the MVBP generate a full support for a particle at a distance $h$ away from the wall ($z = 0.01$ m) and does treat particles located at $2h \geq |\mathbf{x}_i - \mathbf{x}_v| > h$ from the wall.
The same trend is demonstrated in Figure 8.11 (c) and (d) which show the zeroth and first moment of the kernel derivative: the eMVBP is almost in near perfect agreement with the theoretical values of the moments whereas, for the MVBP and VBP the missing support for particles $2h \geq |x_i - x_v| > h$ away from the wall generates a non-negligible error. For the particle $h \geq |x_i - x_v|$ away from the wall the moment is recovered but deviates from the theoretical value since the support edge particles are missing.

As the simulation advances in time to 5 seconds, results of the pressure and density are plotted in Figure 8.12 (a) and (b). The pressure prediction for the eMVBP is in closer agreement to the analytical hydrostatic pressure especially near the boundary. The small discrepancy from the hydrostatic pressure in the interior domain is due to the Shepard filter in both the MVBP and the eMVBP.

![Figure 8.12](image)

Figure 8.12. Still water test case: (a) hydrostatic pressure and (b) density after 5 seconds at a cross-section in the middle of the domain and comparison with the analytical solution for all three methods.
Figure 8.13. Particle distribution and pressure field at 5.0 seconds for the (a) MVBP and (b) eMVBP.

Figure 8.13 shows the particle distribution where the particles are coloured according to pressure for the MVBP and eMVBP at $t = 5$ seconds. It can be observed that the interior fluid domain has deviated from the uniform stencil. The VBP is not shown since severe penetration took place. The observed particle self-redistribution is generated by the SPH interpolation error both at the boundary and inside the fluid domain and it has already been analysed in literature [42, 108]. Note, that this phenomenon is different from the spurious particle recirculation close to the boundary is observed when a kernel truncation effect is present close to the boundaries [45, 152].

The error deviation from the analytical hydrostatic pressure for the VBP, MVBP and eMVBP are 52.74%, 3.53% and 0.15% respectively. The eMVBP method generated a more regular fluid particle distribution near the boundary (particularly in the corners), and this is due to the local uniform support of the boundary, where the fluid particles rearrange themselves locally in a uniform stencil with respect to fictitious particles.

In Figure 8.14 the zeroth and first order moments of the kernel and its derivatives are plotted. Significant improvements are observed for the zeroth moment next to the boundaries in comparison to the VBP and MVBP. For the first moment of the kernel, improvements are also observed. The moments of the derivative of the kernel also exhibit satisfactory improvements with both moments showing nearly linear behaviour near the boundary. This is due to the fact that, the fictitious particles are always uniformly distributed and this helps to reduce the error in the SPH interpolation regardless of the fluid particle distribution. Also the
local interpolation for density will result in a localised pressure that is hydrostatic with respect to the density of the fluid particle itself.

Concluding, the eMVBP generates a uniform kernel particle distribution within the boundary region that evidently reduces the error in the kernel moments and derivative which at best, with uniform interior particle stencil can be approximately $C^1$ consistent. That is not only applicable to the nearest particle to the wall but for all particles within $2h$ i.e. the second row of interior particles located at $z = 0.15$ m in the Figure 8.11 and Figure 8.14.

![Graphs showing kernel moments and derivatives](image)

Figure 8.14. Still water at time 5 seconds: (a) zeroth and (b) first moment of the kernel, (c) zeroth and (d) first moment of the kernel derivative in $z$ direction.

### 8.4.2. Wedge in a tank

A more elaborate test case that includes internal and external angles in addition to slope boundaries is a still water tank with a wedge. Researchers [58, 143, 197] in the past have used
this case in static and dynamic conditions to check the ability of the schemes to reproduce pressure under gravity, a necessary requirement for the Navier-Stokes equations. The geometry of the test case is a tank of 4 m length and height containing a wedge of $3\pi/2$ radians angle at the centre with a height of 1 m containing water of height $H = 2.025$ m. The particle spacing for this case was set to $\Delta x = 0.05$ m resulting in 3548 fluid particles. The artificial viscosity was set to $\alpha_p = 0.1$ with a Shepard filter every 30 time steps. Particles were initialized with zero velocity and hydrostatic pressure.

![Figure 8.15. Particle distribution and different particle arrangements for the tank with a wedge, uniform stencil (●), staggered stencil (●), non-uniform with respect to the wall (●) and sampling cross-section area.](image)

To demonstrate the ability of the proposed modifications to deal with any particle distribution mechanism, a number of different initial particle arrangements have been used. A non-uniform stencil and staggered particle arrangements can influence behaviour since the non-uniform stencil at the boundary tends to generate more error. Therefore, a uniform initial particle arrangement is located in the upper interior domain where there is no special interest in the geometry, a staggered initial arrangement in the vicinity of the wedge and a staggered but with varying distances of particles to the wall of the tank (non-uniform with respect to the wall). In addition, this configuration demonstrates the ability of the treatment to re-organise the fluid particles near the wall with a uniform stencil for the image particles $k$. Figure 8.15 shows the particle distribution zones.

First, the moments of the kernel and its derivative are investigated. The results at steady state are presented in Figure 8.16 for a cross-section at the beginning of the wedge slope after the $2\pi/3$ radians corner shown in Figure 8.15.
Evidently, the zeroth moment of the kernel shows some more accurate results for the eMVBP near the wall with values close to the full theoretical value of the support kernel. The first moment of the kernel has marginal improvement. However, the zeroth and first moment of the kernel derivative in the $z$ direction have drastically improved in comparison with the VBP and MVBP, near the wall. Note the zeroth moment and the first moment’s derivative are expected to be less than the theoretical value due to the kernel support at the free surface ($z = 2.028$ m).

\begin{figure}[h]
\centering
\subfloat{\includegraphics[width=0.45\textwidth]{fig8_16a.png}} \hspace{0.05\textwidth} \subfloat{\includegraphics[width=0.45\textwidth]{fig8_16b.png}}
\subfloat{\includegraphics[width=0.45\textwidth]{fig8_16c.png}} \hspace{0.05\textwidth} \subfloat{\includegraphics[width=0.45\textwidth]{fig8_16d.png}}
\caption{Wedge in a tank at time 15 seconds: (a) zeroth and (b) first moment of the kernel, (c) zeroth and (d) first moment of the kernel derivative in $z$ direction.}
\end{figure}

In hydrostatic conditions and especially using large internal and external corners, many wall boundary conditions fail to recover the pressure and exhibit parasitic velocities in the interior domain that propagate from the boundaries [58].
Figure 8.17 shows the pressure field for all interior particles of the domain. It is evident that scattering of the pressure near the boundary is severe for the VBP with high pressures near the boundary and the interior domain. The MVBP shows some improvements but particles near the wall exhibit deviations from the analytical hydrostatic pressure. Scatter pressure is observed up to the height of 1 m, equal with the height of the wedge. The eMVBP pressure shows significantly better behaviour near the wall and wedge with a reduction in the scattering of pressure. The unphysical deviation in the interior domain is due to the use of the Shepard filter which is necessary for a uniform pressure distribution. \( \delta \text{-SPH} \) or other advanced viscous models have not been used, since the interest with the actual interpolation error and not the pressure corrections.

The non-dimensional velocity field \( u_z / \sqrt{g H} \) for the \( z \) direction for the interior particles at \( t = 20 \) s is plotted in Figure 8.18. Both the VBP and MVBP show large velocity fluctuations not only near the boundary but within the interior domain as the velocity fluctuations are propagating to the interior domain. The eMVBP velocity field shows better behaviour with minimal velocity fluctuations especially near the wall boundaries with a maximum variation less than 0.2\% of \( \sqrt{g H} \) where the latter method’s range is 0.8\%. The Reynolds number for this simulation can be estimated as 138 where \( Re = (g H)^{1/2} H/\nu \) and for weakly compressible flows \( \nu = (ahc)/8 \) [152]. Ferrand et al. [58] performed a similar case but with a less challenging angle of 90° and lower \( Re = 110 \) obtaining maximum variations of 0.002\% of \( \sqrt{g H} \).

At this point it should be mentioned that with the VBP and MVBP penetration of the wall boundary had occurred at 20 seconds. The results presented herein exclude the penetrated fluid particles since their properties are unphysical. Figure 8.19 shows the particle distribution and pressure at the left corner. The eMVBP has an orderly uniform particle distribution. The VBP and MVBP figures show a pressure wave originating from the corner and propagating to the interior domain. Also, an unphysical gap exists between the first two rows of particles adjacent to the boundary since neither the VBP nor MVBP methods complete the support of particles located within a \( 2h \geq x > h \) distance from the wall. For the eMVBP the pressure field is hydrostatically orientated and the unphysical gap is negligible.
Figure 8.17. Wedge in a tank at time 20 seconds: pressure field distribution for the interior fluid domain for the (a) VBP, (b) MVBP (b) and (c) eMVBP.
Figure 8.18. Wedge in a tank at time 20 seconds: velocity field distribution for the interior fluid domain for the (a) VBP, (b) MVBP (b) and (c) eMVBP.
Figure 8.19. Wedge in a tank at time 20 seconds: pressure field and particle distribution for the interior fluid domain at the left corner for the (a) VBP, (b) MVBP (b) and (c) eMVBP.

8.4.3. Tangential annular flow

An interesting dynamic case is the tangential annular flow also known as axisymmetric Couette flow. This test case consists of two coaxial and co-rotating cylinders rotating at a fixed frequency of 0.05 Hz as shown in Figure 8.20. The outer circle has a diameter of 1 m and the inner circle a diameter of 0.2 m. The region between the circles is filled with a fluid with particle size equal to 0.05 m using an artificial viscosity of $\alpha_\pi = 0.1$. The gravity force is neglected and a Shepard filter is applied every 30 time steps. The pressure and the velocity at the beginning of the simulation are set to zero with only the density of the fluid particles set at $\rho_0 = 1000$ kg/m$^3$. A schematic is shown in Figure 8.20.
At steady state, only the tangential velocity is non zero with no pressure gradient in the circumferential direction, the velocity profile can be found as [18]

\[ u_\theta = \frac{C_1 r}{2} + \frac{C_2}{r}, \quad (8.27) \]

where \( C_1 \) and \( C_2 \) are the integration constants. The tangential annular flow is a challenging test case where penetration can occur at the inner and outer circle due to the radial pressure gradient that needs to be balanced by the boundary. The velocity distribution at steady state is a good measure of the effectiveness of the method compared with the analytical solution. In addition the performance of the eMVBP over curved and moving boundaries can be examined.

Figure 8.21 shows the analytical solution for the tangential velocities where only the eMVBP approach shows close agreement of the numerical results, with the analytical solution close to the outer circle and the interior domain. There is a small deviation near the inner circle for the fluid particles adjacent to the wall which will be discussed shortly.
Figure 8.21. Tangential velocity field of the radial direction for the VBP, MVBP and eMVBP methods at time $t = 15$ s.

Figure 8.22 shows the moments and the gradients obtained after 15 seconds of simulation. The radius of the inner and outer circle has been purposefully chosen to highlight the curvature limitation of the method.

First the outer circle at $R_1 = 1.0$ m is examined. The moments of the kernel for the eMVBP show marginal improvement over the VBP and MVBP. This is expected with the fictitious stencils generated being virtually identical. For the derivative of the kernel, which is more sensitive to the partition of unity condition and particle disorder, the results are improved. Nevertheless, the $M'_1$ shows some deviations from the theoretical value.

If we consider the generation mechanism for a single particle over a straight wall as before, the fictitious particles generation will be uniform with respect to the fluid particle since the virtual particles are arranged on the wall. Now, if the same is repeated for a curved wall, the virtual particles that are located along the wall line will generate fictitious particles which resemble the curved wall geometry. Therefore, the fictitious particles have deviated from the uniform particle distribution with respect to the fluid particle, thus introducing error to the first moment of Equations (8.8) and (8.9).
Figure 8.23 demonstrates the generation mechanism near the curved boundaries of the two circles. The outer circle tends to spread the fictitious particles with a small deviation from a uniform stencil whereas the inner circle, where the curvature is larger, deviates greatly from the uniform stencil and fictitious particles are packed closer together. This is confirmed by the moments of the inner circle of Figure 8.22. It is clearly shown that at $R_2 = 0.2$ m the error of the eMVBp is significant. Similar discrepancies are shown in the tangential velocity field of the interior domain in Figure 8.21. Unfortunately this is an inherent weakness of the generation mechanism that has been identified for large curvatures and is currently under further development.

![Diagram](image)

Figure 8.22. The zeroth and first moment for the kernel (a), (b) and its derivative (c), (d) at $t = 15$ s in the radial direction.
Figure 8.23. Particle generation mechanism for the two circles, outer (a) and inner (b) circle. Note the spacing of the inner circle fictitious particles distribution in respect with the outer fictitious particle.

8.4.4. Dam break

Herein, the classical SPH dam break dynamic case is examined. The dam break is a popular test case for SPH due to its non-linear violent flow field, free surface and impact flow [44]. Wall discretization in classical SPH models suffers from fluid particles sticking on the boundary, large unphysical gaps between the boundary and the fluid [45], pressure fluctuations and unphysical large repulsive forces [152] after impact of the water toe advance at the wall.

The geometrical configuration of the test case and the results are in line with [105]. More specifically, the water height was set to $H = 2L$ and the water column length $L = 1$ m as shown in Figure 7.4. For this test case the artificial viscosity was set to $\alpha_s = 0.1$. Three different particle spacing have been used to examine the convergence of the height reduction and toe advancement as the dam breaks in comparison with the experimental result. The particle spacing $\Delta x$ are 0.05, 0.025 and 0.0125 m resulting in 1280, 4160 and 14721 particles respectively.

Figure 8.24 shows the qualitative results for the velocity and the pressure field of a dam break at $t = 0.6$ s and $t = 0.9$ s, before and after the water toe has impacted the wall. At $t = 0.6$ s the velocity and pressure show smooth fields without spurious behaviour and most importantly no particles sticking on the left wall as the height of the water column declines. Also the water toe does not exhibit unphysical gaps between the boundary and the fluid. At $t = 0.9$ s the toe has impacted the wall. As before, the velocity and pressure fields are smooth and no
particle penetration has occurred. In addition, no evidence of unphysical high repulsive forces is present on the right wall where the fluid has impacted the wall.

Figure 8.24. Dam Break: velocity and pressure field of the dam break at $t = 0.6$ s and $t = 0.9$ s for particle spacing of $\Delta x = 0.0125$ m.
Figure 8.25. Dam Break: dimensionless toe (a) and height advance (b) of water convergence study for 3 different particle spacing.

A convergence study is also performed with numerical results for three different particle spacing compared with the experimental results of [105] shown in Figure 8.25. The non-dimensional $t^*$, $x^*$ and $h^*$ are defined in Equation (7.1). All three configurations show good agreement with the experimental results. In addition, improvements due to particle spacing are only evident for the first two configurations showing that convergence can be achieved with low particle resolution since the error propagation to the interior domain from the boundary is reduced.

**Partial Conclusions**

In this Section, a novel method to enforce closed boundary conditions in SPH schemes has been presented. The capability to discretize arbitrarily complex 2-D geometries and to assure approximate zero and first order consistency is obtained using a local point symmetry approach. The numerical scheme has been tested simulating the water at rest for domains with different shapes. Finally a dam break test case has been reproduced to validate the scheme also for fast dynamic flows. Numerical results showed significant improvement over the former methods for the kernel moments and the derivative of the kernel. The method presented is able to address many drawbacks of popular ghost-type particle methods such as particle penetrations, large distance between boundaries and fluid particles, unphysical large pressure values at the boundary particles. Also the pressure and velocity fields showed important improvement. Further development should be conducted for large curvature even though the results were acceptable.
8.5. Wall Boundary conditions extension to 3-D

8.5.1. Wall representation using triangles

In previous local virtual boundary particle methods such as the MVBP and the eMVBP [59, 63, 197], the solid boundary was represented by virtual boundary particles that were used only for geometrical purposes, i.e. the generation of a set of fictitious particles within the truncated kernel support of the boundary. An example is shown in Figure 8.7.

Such an approach can be cumbersome in 3-D especially near corners. Moreover, each fluid particle interacts with all the virtual particles within its support and large numbers of virtual particles are required to represent the solid boundary in 3-D which increases computational cost. A different approach is now presented where the solid boundary is represented using surfaces comprised of triangles. The triangulated surfaces can be readily used in 3-D without special treatments when discretizing arbitrary complex geometries. Therefore, the fully uniform fictitious stencil is translated according to the position of the fluid particle and the triangulated area as shown in Figure 8.26 using a Ray casting algorithm [177]. Consequently, each fluid particle interacts only with triangles located within its support reducing the interaction drastically. The mechanism used to complete the truncated support near the boundary using the triangles is described next.

![Figure 8.26. Local uniform stencil generation using triangulated surfaces in 3-D.](image)
8.5.2. Local uniform stencil boundary condition (LUST)

Vacondio *et al.* [197] using the eMVBP and Fourtakas *et al.* [63] using the eMVBP of Section 8.3, proposed a 2-D wall boundary condition method based on a local point symmetry. They used the virtual particles to generate a set of uniform fictitious particles to complete the truncated kernel support depending on the distance of the fluid particle to the solid boundary and thus maintain zeroth and first order consistency, approximately. Generally, fictitious particles were generated as

\[ x_{k,1} = 2n_{smt}x_v - x_i \]
\[ \vdots \]
\[ x_{k,n_{smt}} = 2n_{smt}x_v - x_i \]  

(8.28)

where the subscripts \( k \) and \( v \) denote the fictitious and virtual particles respectively and \( n_{smt} \) is the number of neighbouring particles within the \( 2h \) radius. Therefore, when a fluid particle approaches the wall, fictitious particles are generated based on the normal distance \( x_{iv}n_v \) from the fluid particle to the wall creating a set of local mirror images of the fluid particle in the boundary at distance \( x_{iv}n_v \) and \( 3x_{iv}n_v \) as shown in Figure 8.7. This method produced satisfactory results for the zeroth and first order moments achieving approximate first order consistency near the boundary [63].

Extension to 3-D is feasible but extra difficulties are encountered when generating a local uniform stencil for 3-D irregular geometries such as corners and curvature. Herein, the aforementioned method has been modified to make it readily extendable to 3-D.

Instead of using virtual particles to generate support for the truncated area of the solid boundary, a complete uniform support is generated at the beginning of the simulation (in 2-D or 3-D) for an arbitrary fluid particle. Since the stencil is uniform the moments of Equations (8.6) and (8.7) in a discrete sense are satisfied and thus the SPH discretization for an arbitrary fluid particle is approximate zeroth and first order consistent. Note that the distances of the fictitious particles are based on the particle initial spacing \( \Delta x \).

When the support of a fluid particle is truncated from the solid wall represented by triangles, the latter arbitrary uniform stencil is applied to the fluid particle. By using the triangulated surfaces, particles that are located within the fluid domain are discarded. The result is a uniform boundary stencil with regularly distributed fictitious particles. The main difference with the eMVBP is that the distance from the fluid to the fictitious particle is constant and
depends on the particle spacing and kernel characteristics only. An example for a fluid particle located at a distance $2\Delta x > x > \Delta x$ and $\Delta x > x > 0$ is shown in Figure 8.27.

![Fluid particle support generation](image)

Figure 8.27. Fluid particle support generation for a particle located at a distance (a) $2\Delta x > x > \Delta x$ and (b) $\Delta x > x > 0$ away from the boundary surface.

As the fluid particle is approaching the wall the sudden inclusion of fictitious particles in the support produces a numerical jump in the interpolation of the fluid particle. To limit the numerical jump in the interpolation an exponential function is used to smooth the influence of the fictitious particles through a buffer zone. In this work the following function has been used

$$f_{bf} = e^{\frac{-1}{\Delta r^2}},$$

(8.29)

where $\Delta r$ is

$$\Delta r = \frac{x_n \cdot n_x}{c_{bf} \Delta x}.$$

(8.30)

The constant $c_{bf}$ determines the size of the buffer zone which in this work is set to 0.5. The mass of the fictitious particle in Equations (8.6) and (8.7) is calculated as follows
\[ m_k = f_{w,f}m_f. \] (8.31)

The buffer created by the exponential function not only smoothes the numerical jump from the inclusion of more fictitious particles in the kernel support but in addition determines the position of the physical solid boundary line to the triangulated surface. Hence, the initial fluid particle distribution is \(0.5\Delta x\) away from the solid surface. A schematic of the buffer zone is shown in Figure 8.28.

![Figure 8.28. Local uniform stencil generation using triangulated surfaces in 3-D.](image)

Similar with the eMVBP, in order to ensure mass conservation and satisfy (8.6) and (8.7), the mass of the fictitious particle is equal to the mass of the interior particle. The density and pressure of the fictitious particle is corrected to compensate for the hydrostatic pressure. By using the Tait’s equation of state, Equation (4.20), the fictitious particle density is assigned according to

\[
\rho_k = \rho_i + \left[ \rho_i \sqrt{\frac{\rho_0 g z \cdot \mathbf{n}_i}{B}} + 1 - \rho_0 \right],
\] (8.32)

where \(B\) is the reference pressure dependent on the numerical speed of sound. The pressure for the fictitious particle is recovered simply by using the EOS in

\[
P_k = B \left( \frac{\rho_k}{\rho_0} \right)^7 - 1.
\] (8.33)

Therefore, in hydrostatic conditions, density and pressure continuity is ensured. One special case exists when a fluid particle is approaching the solid boundary with a density smaller than
the reference density. In this case the pressure of the fictitious particle is arbitrarily set to zero until the fluid density increases to the reference density. The reasoning behind this artificial limit is to ensure the fictitious particles only exert repulsive forces and ensure density conservation for the approaching particle as it enters and leaves the $2h$ region.

Finally, the velocity of the fictitious particle uses the velocity mirroring approach by Takeda et al. [192]

$$
\mathbf{u}_k = (\mathbf{u}_j - \mathbf{u}_y) \frac{\mathbf{x}_{sk}}{\| \mathbf{x}_{sk} \|} - \mathbf{u}_y.
$$

Concluding, the solid boundary in this work is represented by triangulated surfaces. When the support of a fluid particle is truncated, thus reducing the consistency of the local approximation, a pre-computed full support is imposed on the fluid particle. Fictitious particles within the domain are discarded and the remaining fictitious particles are used in the fluid interpolation by applying a local hydrostatic correction to the density and pressure. If the interior domain particles are uniform then the approximation is first order consistent. Otherwise the interpolation consistency is reduced due to the interior domain disorder. Overall, the fictitious particle uniform stencil is said to be approximately first order consistent.

### 8.5.3. Numerical Implementation on GPUs

As explained in the previous Section 8.5.2 each fluid particle has a predefined stencil of fictitious particles with whom to interact. This predefined stencil does not change during the simulation so it is created and stored in the GPU memory at the beginning. When computing acceleration for a fluid particle located close to the wall, a test should be performed to determine which fictitious particles in the predefined stencil belong to the boundary region.

To determine whether a fictitious particle lies in the boundary region it is necessary to check if the line segment connecting the fluid particle to the fictitious one intersects any of the triangles that define the wall. If so, the triangle with the intersection point closest to the fluid particle is chosen. When several triangles are intersected, the ray casting algorithm [177] in 3-D is used to determine if the fictitious particle is valid, whether it lies in the boundary region or not. The number of triangles required to define complex geometries can be high. In order to reduce the number of triangles included in the test the neighbour-list algorithm used for the simulation is altered to include a list of triangle neighbours in each neighbour-list cell. The list of triangles in each cell can be created and stored in the GPU memory at the
beginning of the simulation. However, the list must be updated if the boundary position undergoes displacement.

Achieving an efficient GPU implementation is a complex task due to multiple loops in the code and memory accesses required to determine the fictitious particles. An option to increase the performance is to store the relevant triangle information in shared memory but the limiting factor is the restricted size of the shared memory (48 Kbytes in current GPUs) referred to as “OneStep”.

Another option to increase the GPU efficiency is splitting the force computation into two steps (“TwoStep”). In the first step the triangle of intersection is determined for each fictitious particle in the predefined stencil. The position of the virtual particle is then computed in the second step and the interaction with the fluid particle is performed. This process significantly reduces the code complexity, decreases the register occupancy and minimizes irregular memory access. Also a combination of the “TwoStep” algorithm with the shared memory is possible. In the latter case, the GPU memory increases since the triangle for each point of the predefined stencil needs to be stored for every fluid particle (“OneStepShared”, “TwoStepShared”). Results for performance and memory usage have been analyzed using a 3-D dam-break impact with obstacle test case. The results are compared with the DBC [44, 46] boundary conditions currently available in the open-source DualSPHysics in Figure 8.29. The DBC is faster than the new approach and the speedup is as seen in the figure. Nevertheless, results obtained in later Section 8.4 with LUST show better agreement with the experimental data.

On the other hand, the LUST boundary condition is also more memory consuming than DBC (Figure 8.30). Figure 8.29 and Figure 8.30 present results for the first version (OneStep), with the improvement when using two steps (TwoStep) and with the use of shared memory (OneStepShared, TwoStepShared).
Figure 8.29. Speed up of DBC over LUST boundary wall boundary conditions.

Figure 8.30. Increasing factor in GPU memory compared to DBC.
8.5.4. Numerical Results

In this Section, the numerical results for two cases are presented beginning with a 3-D still water case with a pyramid in the centre which demonstrates the ability of the method to deal with sharp corners and irregular shapes and maintain approximate first order consistency near the boundary. Also, the pressure and velocity field is assessed. This is followed by the SPH benchmark test case 2 which is used to validate the kinematics of the proposed LUST boundary condition.

8.5.4.1. 3-D Still water with a pyramid

A 3-D still water tank with dimensions of 1 x 1 x 1 m encloses a trigonal pyramid in the bottom centre of the tank with a height of 0.25 m and equilateral triangle faces. The tank contains water with a height of 0.5 m and particle spacing $\Delta x$ of 0.0117 m resulting to 307317 fluid particles.

The initial density of the fluid is initialised to hydrostatic conditions with an artificial viscosity of $a_\pi = 0.1$ and the diffusion parameter $a_d$ of $\delta$-SPH set to 0.1. The kernel smoothing coefficient is set to $a_{smt} = 1.3$.

Figure 8.31. Cross-section of the 3-D still water case with a pyramid.
The case has been chosen to demonstrate the ability of the LUST BC to maintain approximate zeroth and first-order accuracy for irregular boundaries and maintain accurate hydrostatic conditions in 3-D. Figure 8.31 shows a cross-section of the case at $x = 0.5$ m. Sampling for our results is taking place at $(x, y) = (0.25, 0.5)$ m using a control volume of $5 \times 5$ particles in the horizontal direction and along the entire depth of the fluid in the vertical direction.

Figure 8.32 compares the pressure predictions of the LUST and DBC with the analytical hydrostatic pressure at $t = 5$ sec. The improvement over the DBC is significant especially near the wall boundary where the DBC performs poorly.

![Figure 8.32. Pressure comparison of the LUST and DBC with the analytical hydrostatic pressure.](image)

A more clear illustration of the improvements in comparison to the DBC can be seen in Figure 8.33 where the pressure fraction error has been plotted for the bottom half of water in the tank ignoring the free surface. The DBC error is close to 40% whereas the LUST BC performs much better with an error less than 5%. A small deviation can be noted in the LUST prediction the cause of which is currently unknown and is the subject of investigation.
Since the operations we perform with the Navier-Stokes equations require the derivative of the kernel, the zeroth and first-order moments of the derivative would be required to achieve first order consistency. Figure 8.34 and Figure 8.35 shows a comparison of the DBC and LUST for the first two moments of the derivative of the kernel. The moments have been plotted in the vertical direction since the flow is hydrostatic.

Similarly to the pressure field the zeroth moment of the derivative of the kernel is greatly improved near the boundary with a maximum value of 1.87 m\(^{-1}\) in contrast to the DBC with a large \(M_0 = 10.36\) m\(^{-1}\). Note that the free-surface moment is 33.67 m\(^{-1}\), thus the moment at the wall for the DBC is reducing the zeroth consistency by a third in comparison with the free surface. Further improvement can be observed for the first moment of the kernel with 0.772 and 0.969 for the DBC and LUST respectively, note that in the case of the first moment the ideal value is 1.0. Since the first moment of the kernel derivative is dependent on particle disorder the uniform particle distribution of the LUST can only improve the approximation of the first moment since the interior domain particles tend to be irregularly distributed, the irregularity means that exact first order consistency is unachievable. Nevertheless, it has been noted that an additional effect of the LUST is that fluid particles near the boundary tend to reorganise their selves into a uniform distribution.
Figure 8.34. Zeroth moment of the kernel derivative comparison of the LUST and DBC.

Figure 8.35. First moment of the kernel derivative comparison of the LUST and DBC.
Concluding, the LUST results show a large improvement in hydrostatic pressure for the 3-D still water over the DBC. Also the kernel derivative moments are again greatly improved in comparison to the DBC with only small variations from their reference values, leading to approximate first order consistency. Next, a dynamic case of a dam break impacting an obstacle is presented.

8.5.4.2. Dam break on an obstacle

We also investigate the reliability of the proposed boundary conditions with a standard free-surface benchmark test for SPH flows, reproducing the SPHERIC Benchmark Test Case #2 as already shown in [44] using DBC. The experiment consists of a 3-D dam break flow impacting with an obstacle. The volume of water is initially confined at one end of the tank in a volume 1.228m long, 1m wide and 0.55m high and is released instantaneously at the start of the simulation.

The initial particle spacing was set to 0.008 m resulting in 1.5 million fluid particles. The initial density of the fluid is initialised to hydrostatic conditions with an artificial viscosity of $a_\pi = 0.01$ and the diffusion parameter $a_d$ of δ-SPH set to 0.1. The kernel smoothing coefficient is set to $a_{smt} = 1.3$.

With the removal of the retaining wall, the fluid floods the dry bed of the tank due to gravity. The experiment provides water heights at different locations (H2, H3, H4) and pressure exerted on the obstacle initially facing towards the water were also sampled to detect water impacts (P1, P2, P3). Figure 8.36 represents the comparison between experimental water heights and SPH numerical values obtained with LUST. The comparison between experimental and numerical pressures is shown in Figure 8.37.
Figure 8.36. Comparison of the experimental water heights at different locations (a) H2, (b) H3 and (c) H4 with the numerical using the LUST BC.
Figure 8.37. Comparison of the experimental pressure exerted on the obstacle at different locations (a) P1, (b) P2 and (c) P3 with the numerical using the LUST BC.
With the removal of the retaining wall, the fluid floods the dry bed of the tank due to gravity. The experiment provides water heights at different locations (H2, H3, H4) and pressure exerted on the obstacle initially facing towards the water were also sampled to detect water impacts (P1, P2, P3). Figure 8.36 represents the comparison between experimental water heights and SPH numerical values obtained with LUST. The comparison between experimental and numerical pressures is shown in Figure 8.37.

The SPH results with LUST reproduce accurately the dam evolution observed in the experiment. Appendix A shows a comparison between the experimental data, results reported by Amicarelli et al. [5] and the results achieved with the LUST BC.

In addition, better results are now obtained with the new boundary conditions in comparison with results obtained with DBC for the same validation case [44] since pressure approximation near the wall is now more accurate and reliable with the new boundary conditions (not show here).

**Partial Conclusions**

In this Section the Local Uniform STencil boundary condition method was presented. The method guarantees approximate zeroth and first order consistency, also for complex geometries in 3-D, by discretizing the surface boundaries by means of a set of triangles. This ensures that no special treatments are required for the corners and no extra particle–interaction loop and thus makes the approach suitable for efficient–parallel simulations.

Still water was simulated in order to investigate the accuracy of the approach, showing that both zeroth and first order moments are approximately reproduced. Finally the simulation of the SPHERIC Benchmark Test Case #2 demonstrated that both water surface elevation and pressures can be accurately simulated.

However, the 2-D and 3-D solid boundary conditions implementation, although derived using the same principle – by completing the kernel support with uniform distributed fictitious particles – have some differences on the particle generation mechanism. The LUST generation mechanism was chosen for the 3-D implementation due to lower memory and arithmetic operations compared with the eMVBP and easier optimisation of the GPU code. However, both approaches can be extended to 3-D.
Chapter 9

9. Conclusions

9.1. General conclusion

In this thesis the development and validation of a Smoothed Particle Hydrodynamics (SPH) multi-phase model has been presented. The multi-phase model focuses on liquid-sediment flows and more specifically the scouring and resuspension of the solid phase by liquid induced rapid flows. The choice of modelling technique in this thesis is based on explicit treatment of the liquid and solid phase using a Newtonian and a non-Newtonian constitutive model respectively that is supplemented by a yield criterion to predict the yielding characteristics of the sediment surface. The Lagrangian nature of smoothed particle hydrodynamics in the absence of a mesh makes the method ideal for complex interfacial and highly non-linear flows. The liquid-sediment flows with scouring and resuspension exhibit phenomena such as a changing interface profile, large deformations and fragmentation of the interface at resuspension of the solid phase.

However, one of the major drawbacks of smoothed particle hydrodynamics is the large computational cost of the method especially for large domains with fine discretization and three dimensional simulations. Naturally, multi-phase models tend to be more expensive than single phase models due to extra arithmetic operations required to resolve both phases with the addition of the mathematical formulation to capture the physical phenomena and the additional memory requirements for storing a second phase in the physical memory of the device. Graphic processing units (GPUs), with massively parallel capabilities have been the choice of hardware acceleration in this work. GPUs’ parallel architecture is well suited to n-body simulations, as discussed in Section 6.2.1, with a sufficient speed up of the SPH algorithms. The open source weakly compressible SPH solver DualSPHysics was chosen as the platform for a CPU/GPU implementation and optimisation of the multi-phase model. The results reported in this thesis show a significant speed up of the algorithm on a single GPU card that is comparable to large high performance computing clusters. The GPU
implementation allowed the simulation of large domains with millions of particles such as the erodible dam break 3-D case which was unfeasible before.

One of the major weaknesses of DualSPHysics and an unsolved issue to date in SPH is the wall boundary conditions that are extrinsic to the SPH formulation. Hence, it was chosen to investigate this area of SPH using a novel method to enforce solid wall boundary conditions in SPH schemes. In this work, the capability to discretize arbitrarily complex 2-D and to some extent 3-D geometries, and to assure approximate zero and first order consistency is obtained using a local point symmetry approach with the objective to reduce the error associated with the discretization of the wall boundary in SPH. Results were satisfactory and a further step was made to extend the method to 3-D using surface triangles in a GPU implementation.

### 9.2. Detailed Conclusions

#### 9.2.1. The multi-phase SPH model

The multi-phase SPH model presented uses the weakly compressible SPH formulation for the liquid and the solid phase by employing the Tait’s equation of state that relates the density to pressure by allowing variations of 1-2% depending on the choice of the numerical speed of sound. The viscous forces in both phases are calculated using a double summation in contrast to other laminar formulations that employ a mixture of finite differences and SPH discretization avoiding the use of the second gradient of the kernel which is troublesome. The advantage of the current implementation is the ease in which constitutive equations can be used. The liquid phase uses the physical dynamic viscosity with the addition of an LES standard Smagorinsky model that is applied to the liquid and yielded sediment phase. In addition, in the liquid phase $\delta$-SPH is used to dissipate large pressure oscillations observed by the use of the stiff equation of state. Results obtained by numerical experiments support the choice of $\delta$-SPH for impact flows where zeroth order filtering is not adequate. Pressure fluctuations and unphysical voids in the liquid phase are treated using a shifting algorithm based on the concentration gradient and Fick’s law of diffusion. Comparison between the original and shifted results validated the use of the shifting algorithm by improving the void formation and the pressure field with sudden impact flows.

The solid phase has been investigated specifically relating to the yield surface and dynamics of the yielded sediment when motion is driven by gravity, pressure and shear forces induced
by the liquid phase. A number of yield criteria have been listed with different characteristics. In this work, the so-called effective stress models have been used mainly due to the WCSPH dependence on the field pressure and their suitability to fully saturated drained sediment conditions that are the focus of this thesis. Numerical experiments demonstrated the Drucker-Prager model to be suitable for the specific cases presented. The yielded surface has been modelled using the Kanatani’s approach that has been used widely in SPH or by more involved non-Newtonian constitutive models such as the Herschel-Bulkley-Papanastasiou model which allows for tuning of the stress growth curve and can behave as a shear thinning or thickening model. Other sub-closure models include the use of the skeleton and pore water pressure in the evaluation of the yield surface with a simplistic seepage force that acts upon the surface of the yielded sediment phase as a dragging force using the velocity of the surrounding particles. Finally, the suspension and entrainment of the sediment is treated using the well known Vand colloidal suspension equation based on the volumetric concentration of the sediment mixture reformulated to provide an apparent viscosity in a Newtonian sense. Comparison between experimental and 2-D numerical results showed reasonable agreement on the interfacial and liquid free surface profiles. Comparison with numerical results by other SPH models showed considerable improvement. However, care should be taken when choosing the model properties as it can lead to over prediction of the scouring profile. Finally, a 3-D case was used to validate the 3-D numerical model. The numerical results were sufficiently close to the experimental data. The size and complexity of the experiments could only have been performed with the use of hardware acceleration and demonstrates the necessity for hardware acceleration and the effectiveness of the GPUs.

9.2.2. GPU Implementation

The necessity for hardware acceleration in SPH has been demonstrated using a complex large scale 3-D experiment. SPH high computational cost is due to the large amount of neighbouring particles in comparison to mesh-based methods. It has been discussed in Section 6.2.1 that the SPH $n$-body nature is well suited to massively parallel architectures. However, the power consumption cost per flop in CPU-based high performance computing clusters has increased dramatically. The scientific community is turning to low power consumption solutions such as the co-processors. GPUs are ideal for $n$-body simulations due to the massively parallel architecture with relative low purchase cost and power consumption per flop. However, the GPU architecture has limited flow control with explicit memory management. Branching and (memory) register occupancy has been at the forefront of GPU
development as a drawback. In this thesis, the multi-phase model was implemented in the CPU and GPU branch of DualSPHysics and a direct comparison yielded a speed up of 58 in comparison with the single-thread serial code. This was achieved by avoiding branching using memory operations that are computational cheap in GPU cards and are mostly hidden by the off-the-chip memory latency. It was noted that the major bottleneck of the code is the “force computation” function. Remedies to speed up the algorithm further is the reduction of the size of the CUDA kernels and the creation of separate linked lists for each phase as reported by other researchers.

9.2.3. Boundary conditions in SPH

In this thesis a new method to impose solid wall boundary conditions in smoothed particle hydrodynamics is presented. The wall is discretized by means of a set of virtual particles and is simulated by a local point symmetry approach. The extension of a previously published Modified Virtual Boundary Particle (MVBP) method guarantees that arbitrarily complex domains can be readily discretized guaranteeing approximate zeroth and first order consistency. To achieve this, three important new modifications are introduced: (i) the complete support is ensured not only for particles within one smoothing length distance, $h$, from the boundary but also for particles located at a distance greater than $h$ but still within the support of the kernel, (ii) for a non-uniform fluid particle distribution the fictitious particles are generated with a uniform stencil (unlike the previous algorithms) which can maintain a uniform shear stress on a particle moving parallel to the wall in a steady flow, (iii) the particle properties (density, mass and velocity) are defined using local point of symmetry to satisfy the hydrostatic conditions and the Cauchy boundary condition for pressure. The extended MVBP (eMVBP) model is demonstrated for cases including hydrostatic conditions for still water in a tank with a wedge and for curved boundaries, where significant improved behaviour is obtained in comparison with the conventional boundary techniques. Finally the capability of the numerical scheme to simulate a dam break simulation is also shown. Numerical results showed significant improvement over the former methods for the kernel moments and the derivative of the kernel. Also the pressure and velocity fields showed important improvement.

Furthermore, the model was extended to 3-D using a more general formulation. Boundary surfaces in the 3-D extension are discretized into sets of triangular planes. Boundary particles are then obtained by translating a full uniform stencil according to the fluid particle position
and applying an efficient ray casting algorithm to select particles inside the fluid domain. The method ensures that a complex geometry can be readily discretized while guaranteeing approximate zeroth and first order consistency. No special treatment for corners and low computational cost make the method ideal for GPU parallelization. Static and dynamic test cases are used to validate the wall boundary model. Significant improvements over the pre-existing wall boundary condition of DualSPHysics have been demonstrated.

9.3. Future work

9.3.1. Alternative Critical state models

The surface yielding of the sediment phase has been based on the effective stress of the sediment skeleton pressure for fully saturated sediment using the Coulomb parameters. However, even if this approach produces satisfactory results it tends to be simplistic. Other critical state models that account for isotropic hardening and softening can be applied. One such model that has received large attention the last two decades is the Cam-Clay model and the extension of it recently by Borja and Ronaldo [22-23]. The Cam clay model is based on the assumption that the soil phase is isotropic, elasto-plastic in a continuous form. The model allows for direct modelling of strain hardening or softening for normally consolidated or over consolidated soil, a non-linear dependence of the volumetric strain on the effective mean stress and limit conditions of ideal plasticity. When using the modified Cam-Clay model the soil is loaded in shear and can be plastically deformed without collapse until reaching the critical state. The soil deforms further in shear under the assumption of ideal plasticity without the change of void ratio and effective pressure. Upon unloading, a linear response of soil is assumed. However, a partly saturated model should be employed either by the approach outlined by Bui et al. [27] or Ulrich et al. [195].

9.3.2. Constitutive modelling using higher order terms

The rheological behaviour of the yielded sediment has been modelled using a non-Newtonian Bingham model based on the Reiner-Rivlin equation [137]. These non-Newtonian models assume isotropic continuous material with the restriction of incompressibility. However in this work only the first order term of the Reiner-Rivlin equation has been applied.

Second order terms for the non-Newtonian Bingham models generally include information for the turbulent nature of the shear layer and suspension layer in terms of the particle
diameter and the concentration of the mixture. Such second order terms for the Herschel-Bulkley-Papanastasiou is the extended Herschel Bulkley model.

Other proposed sophisticated non-Newtonian models such as the Generalised Viscoplastic Fluid model (GVF) (Chen et al. [35]) use a second-order term based on flow behaviour indices that can be adjusted for the specific soil type and application. However, as discussed earlier tuning of these parameters can be cumbersome.

At the sediment surface, the application of a fluvial model such as the Shields parameter in combination with a semi-empirical formulation (see van Rijn [200-201]) could be advantageous as reported by Manenti et al. [138]. However, resolving the turbulence at the surface is important with the use of such models [67]. In this work, the turbulence characteristics of the flow were not investigated rigorously. However, using a fluvial model, at the surface the turbulence should be resolved either using a log law or by increasing the particle resolution to capture the turbulent effect. The Shields parameter criterion should complement the yielding and plastic flow characteristics of the sediment phase. The resulting model could be applicable to application with rapid impact and fluvial flows including river and sea bed erosion.

9.3.3. Multi-GPU implementation

The need for hardware acceleration in SPH and the reasoning behind the use of GPUs has been discussed in many sections of this work. On the other hand, GPU cards are limited in terms of scalability and physical memory. It has been noted that the GPU implementation scalability level out at around 1.5 million particles. In addition, the finite and non-expandable memory in GPU cards forces an upper limit on the particle resolution. This leads to the development of multi-GPU codes such as the work of Dominguez et al. [53] using DualSPHysics. In the work by Dominguez et al. several GPU cards were interconnected through MPI to form a massively parallel cluster that is only limited to the amount of cards to be used and the MPI communication speed that is used for data exchange.

Such an approach could be beneficial especially for multi-phase simulations where memory and arithmetic operations are in a premium. Also, more advance numerical models mentioned in the Section could be implemented without the consideration of excessive computational cost that can impact the particle spacing, i.e. lower resolution or enhanced physical modelling.
9.3.4. Future applications and developments

Future applications of the numerical liquid-sediment model can be extended to debris flows and subaqueous mass movements. Subaqueous debris flow has been modelled traditionally by visco-plastic models where the water reduces the strength of the sediment bed and increases the viscous behaviour of the sediment [129]. A dominant feature of subaqueous debris flow is the turbidity currents generated by the mixing of the water directly into the body of the flow behind the front run-off area [80]. The incorporation of a fluvial model such as the Shields parameter and a Reynolds average turbulence model to avoid under resolving by the LES model could potentially capture the turbidity currents. In addition, the sediment mixture phase could be modelled using ISPH to increase the accuracy of the lithostatic pressure evaluation in the solid, in a similar approach to Bui et al. [25]. The subaqueous mass movements could also be applied to tsunami generation mechanisms.

The introduction of heat effects may be advantageous to industrial applications such as the current nuclear application or metal forming and die-casting applications.
Appendix A

3-D dam break on an obstacle

Herein, comparisons of the results reported by Amicarelli et al. [5] for the 3-D dam break over an obstacle test case are shown. Figure A.1 (a-b) shows a comparison of the experimental data with the results of Amicarelli et al. [5] and the LUST wall boundary treatment at location H2.

The water height, pressure and time is shown in non-dimensional form by

\[
H = \frac{H_{\text{probe}}}{H_{\text{max}}}, \quad C_p = \frac{P}{1/2 \rho U_{\text{ref}}^2}, \quad T = \frac{t}{t_{\text{max}}},
\]

(A.1)

where \( H_{\text{probe}} \) is the probe height measurement, \( H_{\text{max}} \) is the initial dam break height, \( U_{\text{ref}} = \sqrt{2gH_{\text{probe}}} \) is the reference velocity and \( t_{\text{max}} \) is the maximum time. The LUST method has good agreement with the results by Amicarelli et al. [5], although there is a slight over prediction for the first reflected wave at around \( T = 0.2 \) to 0.4. However the LUST method captures adequately the second incoming wave at around \( T = 0.8 \).

Figure A.2 (a-b) to Figure A.3 (a-b) compares the experimental data with the results of Amicarelli et al. [5] for pressure probes P1 and P3. The results obtained by the LUST BC show good agreement for both pressure probes and more specifically probe P1 perform better that the results reported by Amicarelli et al. [5] and the semi-analytical BC. Similarly pressure probe P2 results of LUST show improvements over the semi-analytical and Amicarelli et al. [5] BC.
Figure A.1. Comparison of results by Amicarelli et al. [5] with the LUST BC of Section 8.5.2 and experimental water heights at location H2 for the dam break over an obstacle test case.
Figure A.2. Experimental pressure exerted on the obstacle at locations P1 and comparison by results reported by Amicarelli et al. [5] and the LUST BC.
Figure A.3. Experimental pressure exerted on the obstacle at locations P2 and comparison by results reported by Amicarelli et al. [5] and the LUST BC.
Bibliography


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